



Sludge Sedimentation Basins Inflow Design Flood Control System Plan

**Clover Power Station
Clover, Virginia**

October 2021

A handwritten signature in black ink that reads "Jonathan Hotstream".

Jonathan Hotstream
Senior Scientist

Prepared For:

Virginia Electric and Power Company
4091 Clover Road
Clover, Virginia 24534

Prepared By:

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50 International Drive, Suite 150
Greenville, South Carolina 29615

A handwritten signature in blue ink that reads "Nakia W. Addison".

Nakia Addison, P.E.
Project Manager



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Revision History

Revision Number	Revision Date	Section Revised	Summary of Revisions
0	10/3/2016		Initial Issue
1	10/14/2021	1, 2, 3, App. A and C	Periodic 5-year revision.

1.0 Background

Virginia Electric and Power Company d/b/a Dominion Virginia Power (Dominion) owns¹ and operates the Clover Power Station. The purpose of this Inflow Design Flood Control System Plan (Plan) is to present the flood control features of the two retrofitted sludge sedimentation basins at the Clover Power Station. These two basins are managed in a manner that controls the inflow design flood, as required by the United States Environmental Protection Agency's (USEPA) final coal combustion residuals (CCR) rule, Title 40 Code of Federal Regulations (40 CFR) Part 257 Subpart D - "Standards for the Disposal of Coal Combustion Residuals in Landfills and Surface Impoundments." The basins are considered existing surface impoundments according to the CCR rule (40 CFR 257.53).

¹ Old Dominion Electric Cooperative owns a 50% undivided interest in the Clover Power Station.

2.0 Inflow Design Flood Control

Hydrologic and hydraulic capacity requirements for CCR surface impoundments are set forth in 40 CFR 257.82. The sludge sedimentation basins are classified as Low Hazard in accordance with 40 CFR 57.73 and 40 CFR 257.74. Based on the Low Hazard potential classification, the CCR unit must adequately manage flow into the unit during and following the peak discharge of the 100-year flood (40 CFR 257.82(a)(3)(iii)).

The sludge sedimentation basins were designed in accordance with 40 CFR 257.82(a)(1), (2) and (3), requiring the CCR units to adequately manage inflows during and following peak discharge and to manage outflows to collect and control peak discharge for a 100-year flood due to the Low Hazard classification.

The Flood Insurance Rate Map for the Clover Power Station (National Flood Insurance Program 2009) indicates that the basins are located in an area determined to be outside the 0.2% annual chance flood, refer to Appendix B. Figures 1 and 2 show the extents of the 100-year flood. The basins are located in an area that is above the 100-year flood elevation; therefore, the basins will not experience surface water inflow during the design flood event. The basins have been designed with several inflow features presented in the sections below to mitigate and control floods.

2.1 Basin Operation

The sludge sedimentation basins are utilized for managing Station low volume process wastewaters. The basins are operated in parallel. Both basins are typically active and available to receive low volume wastewaters from the Station. As needed, accumulated solids are dewatered, removed, and transported to the Station's CCR landfill for disposal. The basins were designed to operate with freeboard, height difference from the top of the berm to the water level, to provide additional short-term storage in the basins.

The water level in the active basin is monitored by level transmitters located in the pump station wet well and during weekly visual inspections. The pump system is programmed to remove water from the basin based on high and low level switches. In addition to level switches, there are high and low level alarms that notify control operators in the Station of needed action.

The following control measures are implemented during basin operation to control the water levels in the basins:

- Operate the basins in parallel during typical operations. The system has the ability to operate the basins independently and isolate each basin for operation, maintenance and cleaning. Based on this use pattern, only one basin is at the operating water level at a time.
- Operate pumps as needed to control the basin water levels.
- Regularly check and maintain grades surrounding the basins to minimize the area contributing to storm water run-on.

2.2 Run-on Control

The basins were designed to control the storm water run-on from a 100-year, 24-hour storm event based upon the Precipitation Frequency Estimates from the National Oceanic and Atmospheric Administration. The storm water run-on volume calculated for the design storm was compared to the storage capacity above the operating water elevation. The evaluation determined that there is sufficient capacity in the basins when operating with 1.5 feet of freeboard, refer to Appendix C.

2.3 Pumping Capacity

The pump station is equipped with two Lawrence VPL3200 pumps with a rated pumping capacity of 410 gallons per minute (gpm) at 110 feet of head. The two pump configuration provides a duty pump and a backup pump in the event of malfunction, maintenance, or repair of the duty pump. A calculation was performed to determine the length of time required to remove the anticipated run-on due to a 100-year, 24-hour storm event, refer to Appendix B. The calculation was performed using only the capacity of the duty pump and resulted in a required time of 51 hours or approximately 2 days to remove the anticipated storm water run-on from both basins. This calculation shows that pumping rates are sufficient in controlling water levels in the basins.

2.4 Conclusions

The basins meet the requirements of 40 CFR 257.82 of adequately controlling the inflows and outflows of peak discharge at the Clover Power Station for the following reasons:

- The basins are located above the 100-year floodplain.
- The basins were adequately designed to manage precipitation volumes resulting from a 100-year, 24-hour storm event.
- The pumping rates are sufficient to control the water levels in both basins.

3.0 Amendment and Periodic Plan Revision

This Plan has been completed in compliance with the requirements set forth in 40 CFR 257.82. This document will be placed in the Station's CCR operating record, posted to the publicly accessible CCR website, and government notifications will be provided.

A periodic inflow design flood control system plan must be prepared every 5 years from the completion date of this Plan. The next Plan update is required by October 2026.

The Plan must be amended whenever the periodic review period is reached or if changes in site conditions occur that will sustainably affect the current written Plan.

4.0 References

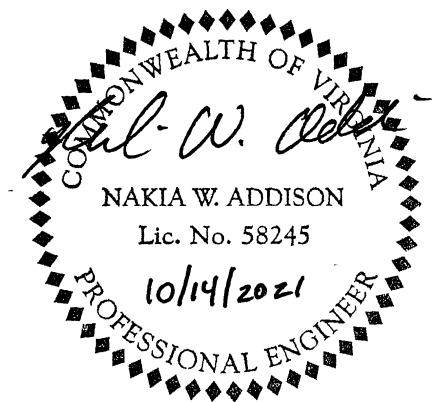
TRC Engineers, Inc. 2021. Initial Hazard Potential Classification – Sludge Sedimentation Basins Clover Power Station. October 2021.

National Flood Insurance Program. 2009. Flood Insurance Rate Map: Halifax County Virginia Panel 350 of 625. Map Number 51083C0350D. Effective Date October 16, 2009. Federal Emergency Management Agency. Washington, D.C.

5.0 Certification

I, the undersigned Virginia Professional Engineer, hereby certify that I am familiar with the technical requirements of 40 CFR 257 Subpart D. I also certify that it is my professional opinion that, to the best of my knowledge, information, and belief, that the information in this demonstration is in accordance with current good and accepted engineering practice(s) and standard(s) and meets the requirements of paragraph (a) of in 40 CFR 257.82.

For the purpose of this document, “certify” and “certification” shall be interpreted and construed to be a “statement of professional opinion.” The certification is understood and intended to be an expression of my professional opinion as a Virginia Licensed Professional Engineer, based upon knowledge, information, and belief. The statement(s) of professional opinion are not and shall not be interpreted or construed to be a guarantee or a warranty of the analysis herein.

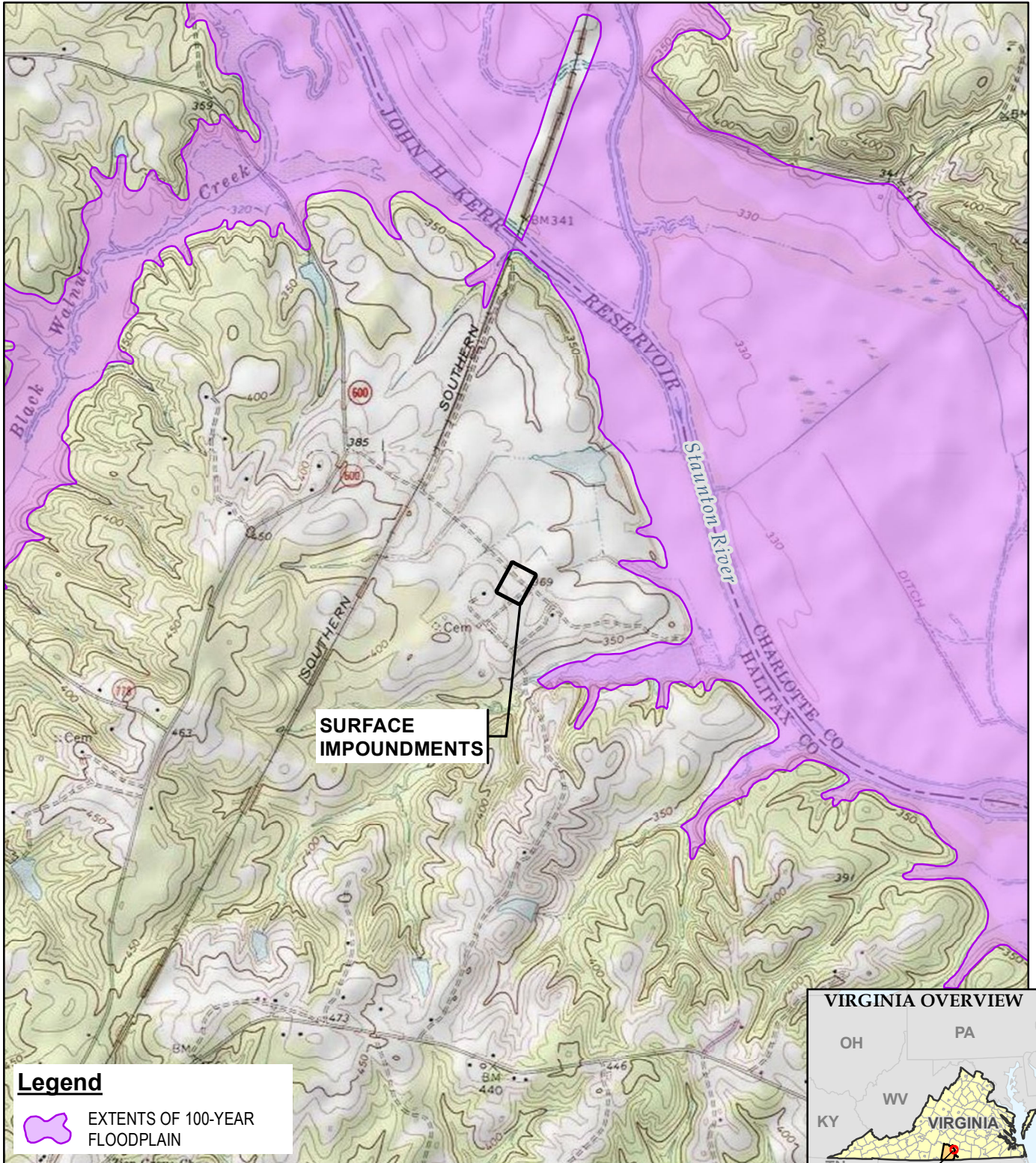


Nakia Addison, P.E.
Printed Name of Professional Engineer

58245
Commonwealth of Virginia License Number


Nakia W. Addison
Signature of Professional Engineer

October 14, 2021
Date



SURFACE IMPOUNDMENTS

Legend

-  EXTENTS OF 100-YEAR FLOODPLAIN

-BASE MAP FROM USGS 7.5 MINUTE TOPOGRAPHIC QUADRANGLE SERIES, 1982.
 -FLOOD DATA ACQUIRED FROM FEMA NATIONAL FLOOD HAZARD LAYER (NFHL).



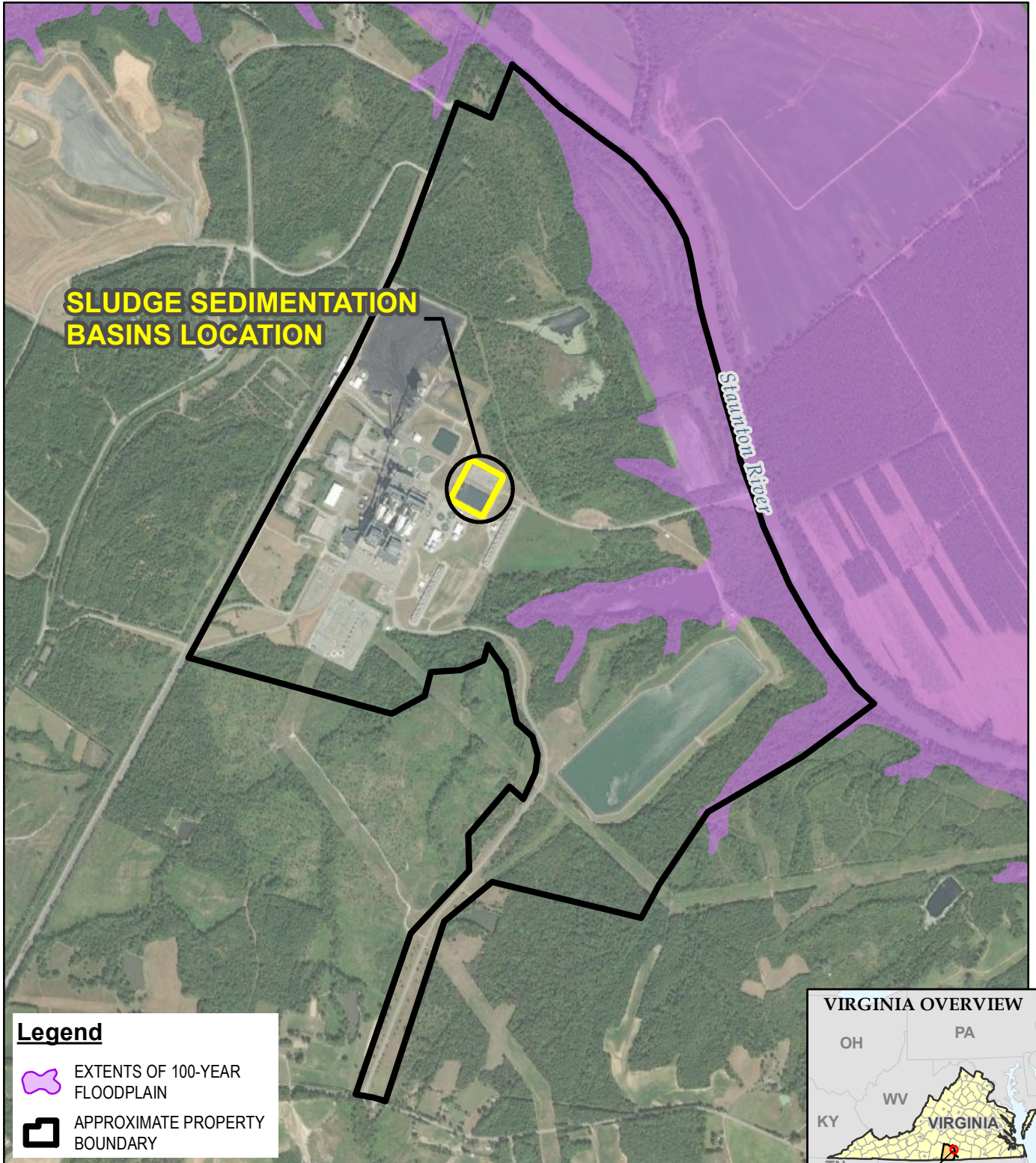
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**DOMINION RESOURCES SERVICES, INC.
 CLOVER POWER STATION
 CLOVER, HALIFAX COUNTY, VIRGINIA**

**INFLOW DESIGN FLOOD CONTROL SYSTEM PLAN
 SITE LOCATION MAP**

DRAWN BY:	A FOJTIK
APPROVED BY:	J. HOTSTREAM
PROJECT NO:	430492
FILE NO.	430492c.mxd
DATE:	OCTOBER 2021

FIGURE 1



-BASE MAP FROM COMMONWEALTH OF VIRGINIA ORTHOPHOTOGRAPHY, 2013.
 -FLOOD DATA ACQUIRED FROM FEMA NATIONAL FLOOD HAZARD LAYER (NFHL).



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DOMINION RESOURCES SERVICES, INC.
CLOVER POWER STATION
CLOVER, HALIFAX COUNTY, VIRGINIA

INFLOW DESIGN FLOOD CONTROL SYSTEM PLAN
SITE OVERVIEW MAP

DRAWN BY:	A FOJTIK
APPROVED BY:	J. HOTSTREAM
PROJECT NO:	430492
FILE NO:	430492d.mxd
DATE:	OCTOBER 2021

FIGURE 2

Appendix A: Selected Retrofit Design Drawings

CLOVER POWER STATION

RETROFIT - SLUDGE SEDIMENTATION BASINS

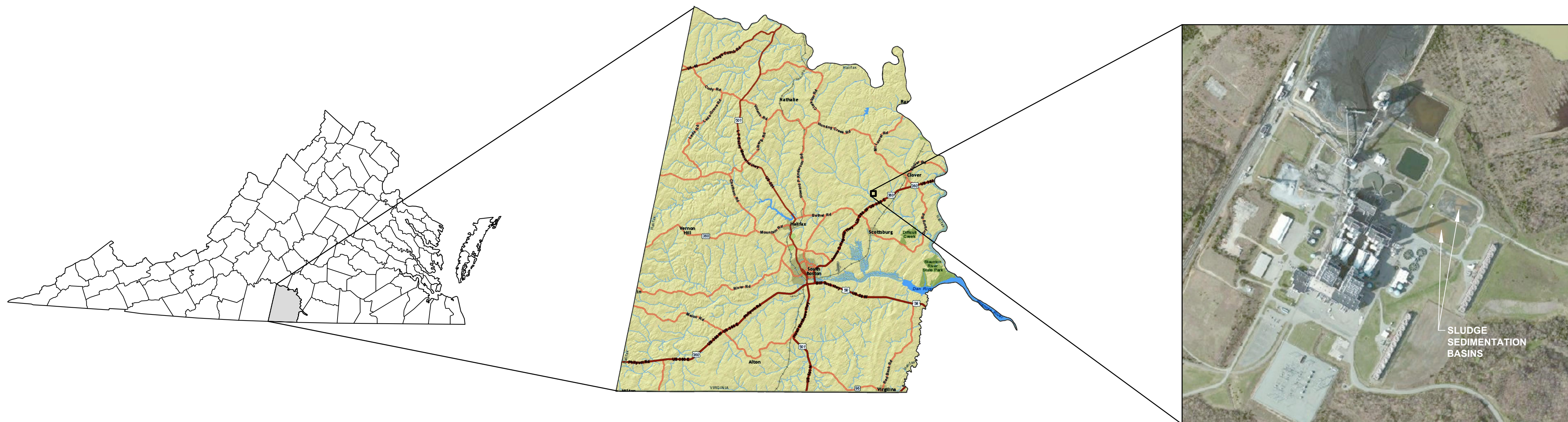
AS-BUILT DRAWINGS

PREPARED FOR: **DOMINION RESOURCES SERVICES, INC.**
CLOVER POWER STATION
CLOVER, VIRGINIA

PREPARED BY: **TRC ENGINEERS, INC.**
GREENVILLE, SOUTH CAROLINA

DATE: **NOVEMBER 2016**
REVISED JULY 2017
REVISED SEPTEMBER 2017
REVISED SEPTEMBER 2019

SHEET INDEX		
SHEET NUMBER	SHEET TITLE	REVISION
66706-15348-CWSG-S3100	TITLE SHEET	
66706-15348-CWSG-S3101	GENERAL NOTES - CIVIL, MECHANICAL, STRUCTURAL	2
66706-15348-CWSG-S3102	GENERAL NOTES - CIVIL, MECHANICAL, STRUCTURAL	1
66706-15348-CWSG-S3103	CIVIL STANDARD LEGEND	5
66706-15348-CWSG-S3104	EXISTING CONDITIONS	3
66706-15348-CWSG-S3105	SUGGESTED CONSTRUCTION SEQUENCE PLAN	5
66706-15348-CWSG-S3106	EROSION AND SEDIMENT CONTROL PLAN	5
66706-15348-CWSG-S3107	OVERVIEW NORTH AND SOUTH BASINS	5
66706-15348-CWSG-S3108	NORTH BASIN LINER SUBBASE GRADES	5
66706-15348-CWSG-S3108B	NORTH BASIN LINER BASE GRADES	5
66706-15348-CWSG-S3108C	NORTH BASIN PANEL LAYOUT	2
66706-15348-CWSG-S3109	NORTH BASIN BASE AND PAVING	5
66706-15348-CWSG-S3110	NORTH BASIN CROSS SECTIONS	2
66706-15348-CWSG-S3111	SOUTH BASIN LINER SUBBASE GRADES	5
66706-15348-CWSG-S3111B	SOUTH BASIN LINER BASE GRADES	5
66706-15348-CWSG-S3111C	SOUTH BASIN PANEL LAYOUT	2
66706-15348-CWSG-S3112	SOUTH BASIN BASE AND PAVING	5
66706-15348-CWSG-S3113	SOUTH BASIN CROSS SECTIONS	3
66706-15348-CWSG-S3114	CIVIL DETAILS - NORTH AND SOUTH BASINS	5
66706-15348-CWSG-S3115	CIVIL DETAILS - NORTH AND SOUTH BASINS	5
66706-15348-CWSG-S3116	CIVIL DETAILS - SOUTH BASIN	3
66706-15348-CWSG-S3117	CIVIL DETAILS - SOUTH BASIN	5
66706-15348-CWSG-S3118	CIVIL DETAILS	5
66706-15348-CWSG-S3119	CIVIL DETAILS - EROSION AND SEDIMENT CONTROL	3
66706-15348-CWSG-M2647	MECHANICAL P&ID	1
66706-15348-CWSG-M4000	YARD PIPING PROFILES	2
66706-15348-CWSG-M4000A	YARD PIPING PROFILES	2
66706-15348-CWSG-M4001	PUMP STATION PLAN VIEW	1
66706-15348-CWSG-M4002	PUMP STATION SECTION VIEWS	1
66706-15348-CWSG-M4003	FLOW SPLIT BOX PLAN AND SECTIONS	1
66706-15348-CWSG-M4004	TEMPORARY PUMP STATION PLAN	1
66706-15348-CWSG-M4005	MECHANICAL DETAILS 1	1
66706-15348-CWSG-M4006	MECHANICAL DETAILS 2	1
66706-15348-CWSG-S6200	STRUCTURAL - SLUDGE PUMP STATION	1
66706-15348-CWSG-S6201	STRUCTURAL - SLUDGE PUMP STATION	1
66706-15348-CWSG-S6202	STRUCTURAL - FLOW SPLIT BOX	1
66706-15348-CWSG-S6203	STRUCTURAL - TYPICAL SECTIONS AND DETAILS	1
66706-15348-CWSG-S6204	STRUCTURAL/ARCHITECTURAL - SECTIONS & DETAILS	1
66706-15348-CWSG-E1001	ELECTRICAL SYMBOLS AND ABBREVIATIONS	1
66706-15348-CWSG-E1002	ELECTRICAL SITE PLAN - DEMO	1
66706-15348-CWSG-E1003	ELECTRICAL SITE PLAN - PROPOSED	1
66706-15348-CWSG-E1004	ELECTRICAL ONELINE	1
66706-15348-CWSG-E1005	ELECTRICAL DETAILS AND ENLARGED PLANS	1
66706-15348-CWSG-E1006	ELECTRICAL DETAILS	1
66706-15348-CWSG-E1007	ELECTRICAL DETAILS AND RACEWAY SCHEDULE	1
66706-15348-CWSG-E1008	CABLE AND PANEL SCHEDULE	1

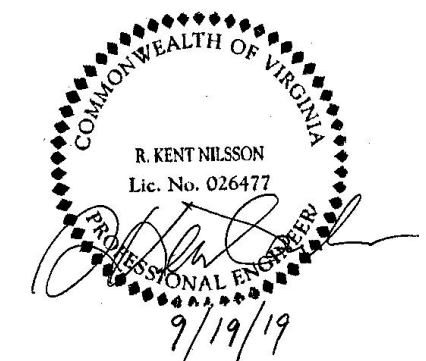


VIRGINIA

HALIFAX COUNTY

SITE LOCATOR

1" = 600'

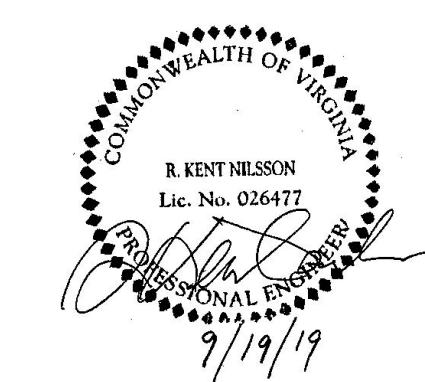


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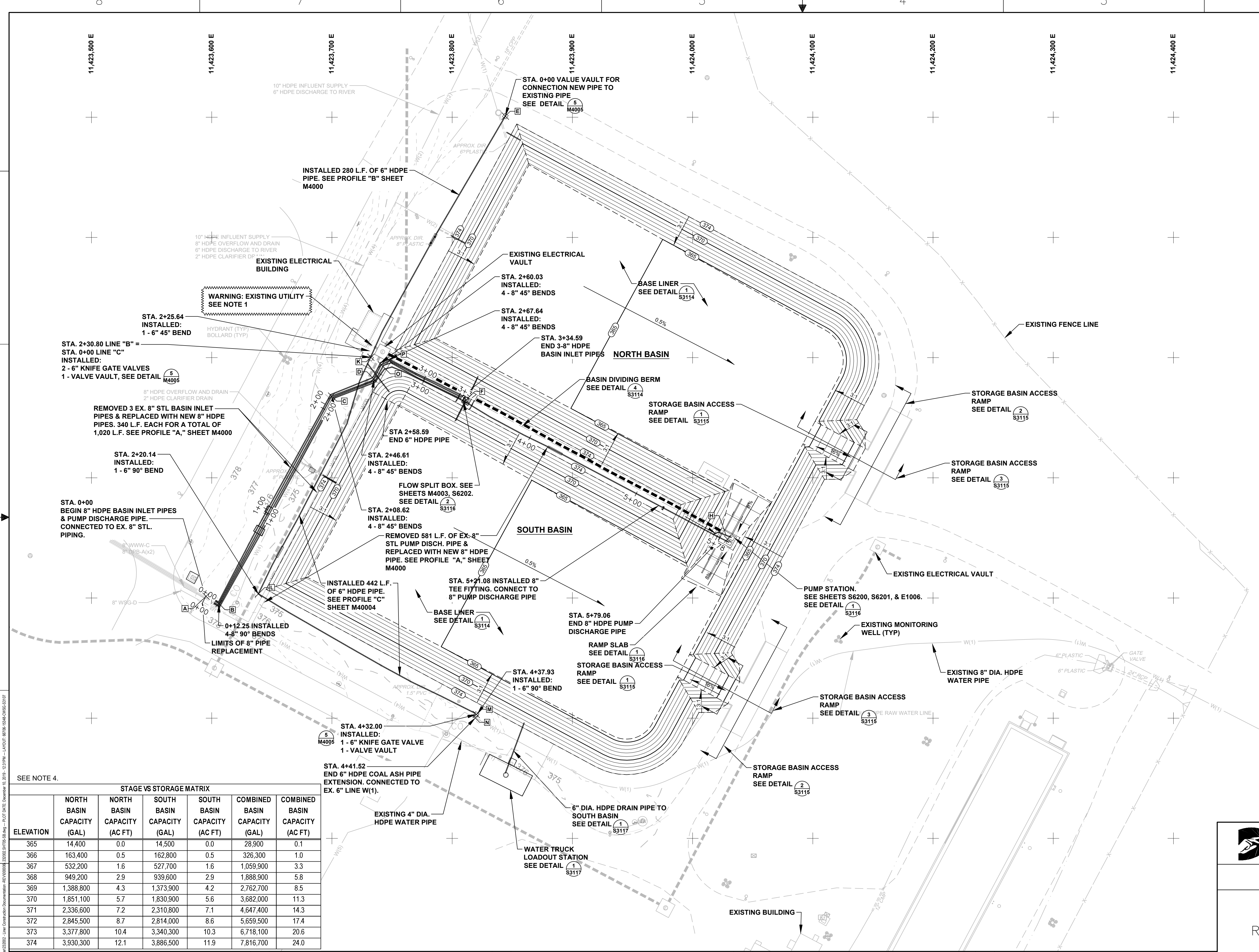
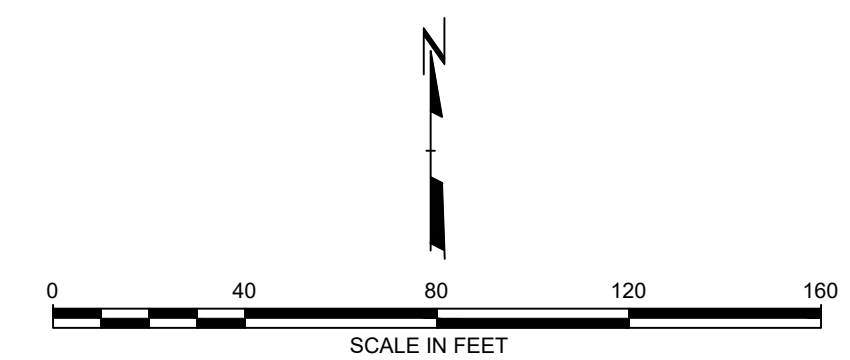


- NOTES**
- REFER TO PLAN SHEET S3103 OF THIS PLAN SET FOR STANDARD LEGEND NOTES AND CONTROL POINT LOCATIONS.
 - GRADES SHOWN ARE BASE GRADES (TOP OF CLAY).
- STATIONING FOR PIPE PROFILES SHOWN ON PLAN SHEET M4000.

- ADDITIONAL NOTES**
- BURIED ELECTRICAL DUCT BANK IN DIVIDER BERM AND NORTHWEST OF BASINS. USE CAUTION DURING EXCAVATION.
 - HDPE YARD PIPING SHALL BE CLASS DR-17 WITH WORKING PRESSURE RATING OF 125 PSI. PIPE AND FITTINGS SHALL MEET THE REQUIREMENTS OF AWWA C906 AND ASTM D2239. PIPE SIZE NOTED INDICATES THE INSIDE DIAMETER.
 - PROVIDE THRUST BLOCKING FOR ALL BURIED PIPE BENDS AS DESCRIBED IN DETAIL 9, SHEET M4006.
 - TABLE PROVIDES VOLUMES BASED ON DESIGN GRADES.



NAD83 CONSTRUCTION COORDINATES				
TAG	OBJECT (CENTERLINE)	NORTHING	EASTING	REFERENCE SHEETS
A	8" PIPE STA. 0+00	3476201.45	11423595.33	M4002
B	8" PIPE STA. 0+12.25 (90° BEND)	3476195.40	11423605.99	M4002
C	8" PIPE STA. 2+08.62 (45° BEND)	3476367.93	11423699.75	M4002
D	8" PIPE STA. 2+46.61 (45° BEND)	3476382.19	11423734.82	M4002
E	6" HDPE VALVE VAULT	3476600.92	11423844.25	M4002, M4005
F	FLOW SPLIT BOX	3476364.65	11423811.46	M4003, S6202
H	PUMP STATION	3476249.01	11424029.00	M4001, M4002, S6200, S6201, E1005
K	6" HDPE VALVE VAULT	3476400.28	11423732.40	M4005
L	6" PIPE STA. 2+20.14 (90° BEND)	3476203.44	11423638.83	M4000A
M	6" HDPE VALVE VAULT	3476103.11	11423820.04	M4005
N	6" HDPE PIPE CONNECTION	3476100.21	11423819.24	M4000A
O	8" PIPE STA. 2+60.03 (45° BEND)	3476394.08	11423741.04	M4002
P	8" PIPE STA. 2+67.64 (45° BEND)	3476397.38	11423747.90	M4002



SEE NOTE 4.

STAGE VS STORAGE MATRIX						
ELEVATION	NORTH BASIN CAPACITY (GAL)	NORTH BASIN CAPACITY (AC FT)	SOUTH BASIN CAPACITY (GAL)	SOUTH BASIN CAPACITY (AC FT)	COMBINED BASIN CAPACITY (GAL)	COMBINED BASIN CAPACITY (AC FT)
365	14,400	0.0	14,500	0.0	28,900	0.1
366	163,400	0.5	162,800	0.5	326,300	1.0
367	532,200	1.6	527,700	1.6	1,059,900	3.3
368	949,200	2.9	939,600	2.9	1,888,900	5.8
369	1,388,800	4.3	1,373,900	4.2	2,762,700	8.5
370	1,851,100	5.7	1,830,900	5.6	3,682,000	11.3
371	2,336,600	7.2	2,310,800	7.1	4,647,400	14.3
372	2,845,500	8.7	2,814,000	8.6	5,659,500	17.4
373	3,377,800	10.4	3,340,300	10.3	6,718,100	20.6
374	3,930,300	12.1	3,886,500	11.9	7,816,700	24.0

5 AS-BUILT	4 NORTH BASIN RETROFIT AS-BUILT	3 UPDATED SUBBASE GRADES AND PIPING REVISIONS	2 UPDATES TO EROSION AND SEDIMENT CONTROL SHEET AND EXISTING CONDITIONS	1 REVISED LINER SYSTEM
09 13 2019	08 01 2018	09 15 2017	07 24 2017	11 16 2016
DM TAF JNH NA RKN CS DS	DM TAF JNH NA RKN CS DS	DM LLS JNH NA RKN	DM LLS JNH NA RKN	DM LLS JNH NA RKN CS DS
CHD DSGN SUPV PROJ ENGR CIVIL ENGR ELEC ENGR MECH ENGR ARCH E.M. ENGR I&C ENGR	CHD DSGN SUPV PROJ ENGR CIVIL ENGR ELEC ENGR MECH ENGR ARCH E.M. ENGR I&C ENGR	CHD DSGN SUPV PROJ ENGR CIVIL ENGR ELEC ENGR MECH ENGR ARCH E.M. ENGR I&C ENGR	CHD DSGN SUPV PROJ ENGR CIVIL ENGR ELEC ENGR MECH ENGR ARCH E.M. ENGR I&C ENGR	CHD DSGN SUPV PROJ ENGR CIVIL ENGR ELEC ENGR MECH ENGR ARCH E.M. ENGR I&C ENGR
REV	REV	REV	REV	REV

**POWER GENERATION ENGINEERING
RICHMOND, VIRGINIA**

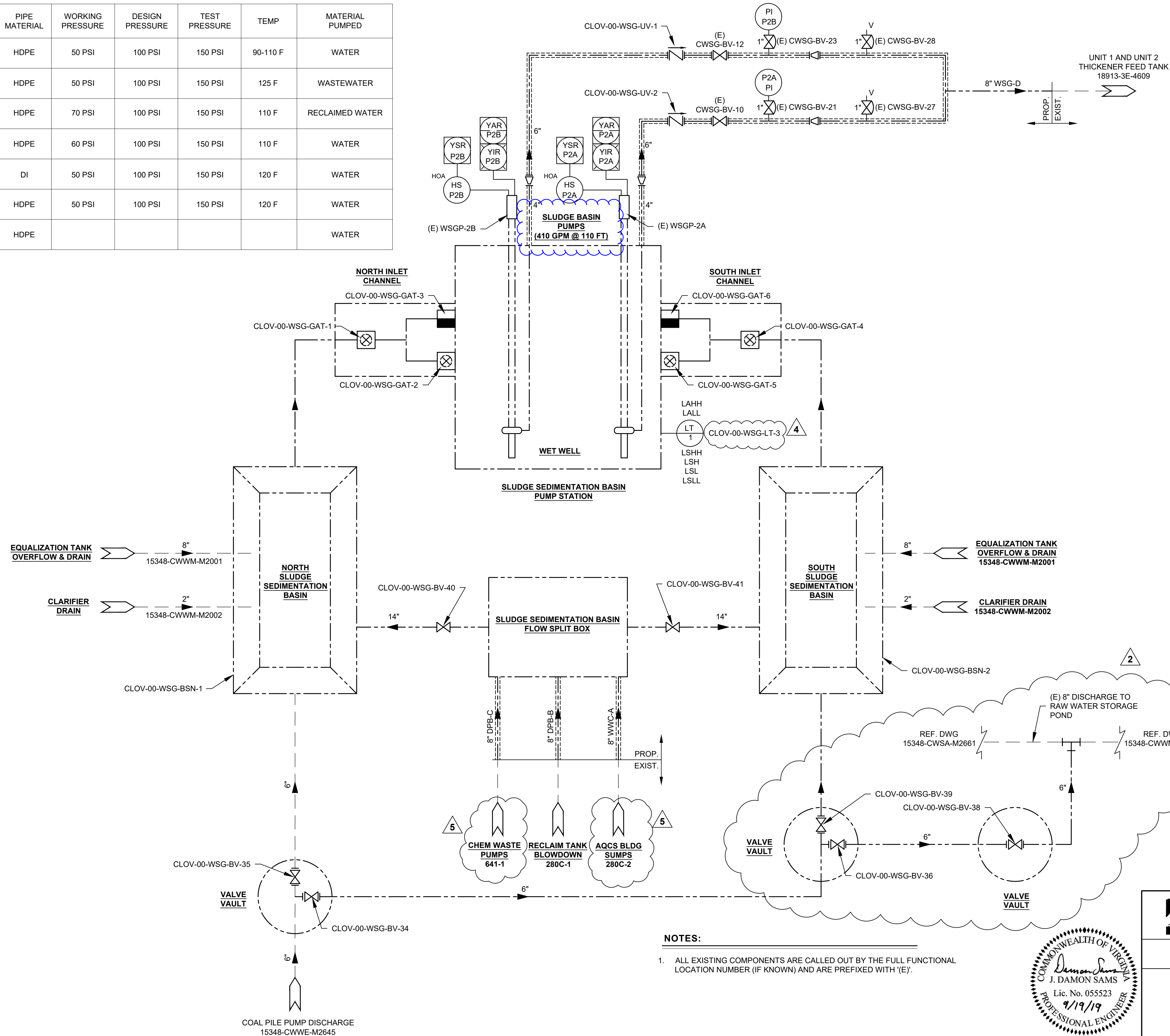
**OVERVIEW NORTH AND SOUTH BASINS
RETROFIT – SLUDGE SEDIMENTATION BASINS
CLOVER, VIRGINIA**

DSGN D.MARSHALL	DSGN SUPV _____	DATE NOVEMBER 2016
DRWN L. STORMER	ENGR SUPV _____	DGNSPEC FOR FILE VERIFICATION
CHD	JH	PROJ ENGR _____
DISPL ENGR	---	SCALE: AS NOTED

UNLESS OTHERWISE NOTED SH 8 OF 41

LINE LIST:

LINE ID	DESC	DIA	PIPE MATERIAL	WORKING PRESSURE	DESIGN PRESSURE	TEST PRESSURE	TEMP	MATERIAL PUMPED
	FROM COAL POND TO SOUTH SLUDGE POND	6	HDPE	50 PSI	100 PSI	150 PSI	90-110 F	WATER
WWC-A	FROM CHEMICAL WASTE	8	HDPE	50 PSI	100 PSI	150 PSI	125 F	WASTEWATER
DPB-B	RECLAIM TANK BLOWDOWN	8	HDPE	70 PSI	100 PSI	150 PSI	110 F	RECLAIMED WATER
DPB-C	AQC BLDG SUMP PUMP DISCH	8	HDPE	60 PSI	100 PSI	150 PSI	110 F	WATER
WSG-D	SLUDGE STORAGE POND PUMP DISCHARGE (ABOVE GROUND)	4/6	DI	50 PSI	100 PSI	150 PSI	120 F	WATER
WSG-D	SLUDGE STORAGE POND PUMP DISCHARGE (BURIED)	8	HDPE	50 PSI	100 PSI	150 PSI	120 F	WATER
	DISCHARGE TO RAW WATER STORAGE POND(BURIED)	8	HDPE					WATER



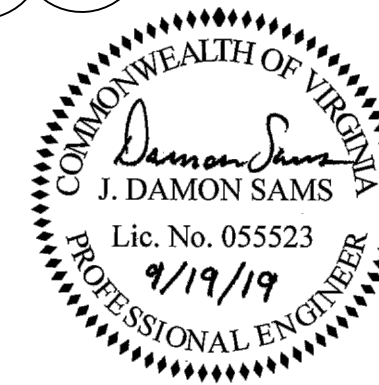
LEGEND AND ABBREVIATIONS

	EXISTING DEVICE		BUTTERFLY VALVE
	FIELD LOCATED DEVICE		MOTORIZED PLUG VALVE
	PANEL MOUNTED DEVICE		CHECK VALVE
	STATUS INDICATOR LIGHT		DIRECTION OF FLOW
	OPERATOR INTERFACE		CAP OR PLUG
	HAND-OFF-AUTO SWITCH		REDUCER
	PROPOSED PROCESS		MIXER
	EXISTING PROCESS		SLIDE GATE
	BUILDING LIMITS		BALL VALVE
	FLOW CONTINUATION ON THIS SHEET		GATE VALVE
	FLOW CONTINUATION ON THIS SHEET		PLUG VALVE
	AIR RELEASE VALVE		DIAPHRAGM VALVE
	SUBMERSIBLE PUMP		WEIR GATE
	CENTRIFUGAL PUMP		MANHOLE
			JACKETED PIPE

AAH	ANALYZER ALARM HIGH	PI	PRESSURE INDICATOR
AAL	ANALYZER ALARM LOW	PIV	PRESSURE INDICATOR-VACUUM
AI	ANALYZER INDICATOR	PSV	PRESSURE SAFETY VALVE
AIT	ANALYZER INDICATOR/TRANSMITTER	TAH	TEMP. ALARM HIGH
ARV	AIR RELEASE/ VACUUM VALVE	TI	TEMP. INDICATE
ASH	ANALYZER SWITCH HIGH	TSH	TEMP. SWITCH HIGH
ASL	ANALYZER SWITCH LOW	TSL	TEMP. SWITCH LOW
AT	ANALYZER TRANSMITTER	VI	VACUUM INDICATOR
FE	FLOW ELEMENT	VT	VACUUM TRANSMITTER
FI	FLOW INDICATOR	YA	STATUS ALARM
HS	HAND SWITCH	YAR	STATUS ALARM RUN
LAHH	LEVEL ALARM HIGH/HIGH	YAS	STATUS ALARM STOP
LSH	LEVEL SWITCH HIGH	YIC	STATUS INDICATE CLOSE
LSHH	LEVEL SWITCH HIGH/HIGH	YIO	STATUS INDICATE OPEN
LSLL	LEVEL SWITCH LOW/LOW	YIR	STATUS INDICATE RUN
LALL	LEVEL ALARM LOW/LOW	YIS	STATUS INDICATE STOP
LSL	LEVEL SWITCH LOW	YSR	STATUS SWITCH RUN
LS	LEVEL SWITCH	ZA	POSITION ALARM
LT	LEVEL TRANSMITTER	ZAC	POSITION ALARM CLOSE
MOV	MOTORIZED VALVE	ZAO	POSITION ALARM OPEN
MS	MOISTURE SENSOR	ZIC	POSITION INDICATE CLOSE
PHIT	pH INDICATOR/TRANSMITTER	ZIO	POSITION INDICATE OPEN
		ZSC	POSITION SWITCH CLOSE
		ZSO	POSITION SWITCH OPEN

NOTES:

- ALL EXISTING COMPONENTS ARE CALLED OUT BY THE FULL FUNCTIONAL LOCATION NUMBER (IF KNOWN) AND ARE PREFIXED WITH '(E)'. REF. DWG 15348-CWSA-M2661 REF. DWG 15348-CWWM-M2003



Dominion POWER GENERATION ENGINEERING RICHMOND, VIRGINIA

TRC

MECHANICAL P&ID

RETROFIT – SLUDGE SEDIMENTATION BASINS CLOVER POWER STATION

REV	DATE	DSGN	DRWN	CHKD	DSGN SUPV	PROJ ENGR	CIVIL ENGR	ELEC ENGR	MECH ENGR	ARCH	E.M. ENGR	I&C ENGR
5	09-13-2019	DM	TAF	JNH	NA	RKN	CS	DS				
4	06-06-2018	DM	TAF	JNH	NA	RKN	CS	DS				
3	08-01-2018	DM	TAF	JNH	NA	RKN	CS	DS				
2	09-15-2017	DM	LLS	JNH	NA	RKN	CS	DS				
1	11-16-2016	DM	LLS	JNH	NA	RKN	CS	DS				

DSGN	DSAMS	DSGN SUPV		DATE	APRIL 2016
DRWN	DSAMS	ENGR SUPV		DGNSPEC FOR FILE VERIFICATION	
CHKD	CBELL	PROJ ENGR		DRAWING NO.	66706-15348-CWSG-M2647
DISPL ENGR	XXX			REV.	2

SCALE: AS NOTED UNLESS OTHERWISE NOTED SH 22 OF 41

Appendix B: Flood Insurance Rate Map

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations (BFEs)** and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0 foot North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations tables in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations tables should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The **projection** used in the preparation of this map was Virginia State Plane South zone. The horizontal datum was NAD 83, GRS80 spheroid. Differences in datum, spheroid, projection or State Plane zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at <http://www.ngs.noaa.gov> or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, NIMS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, Maryland 20910-3282
(301) 713-3242

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242, or visit its website at <http://www.ngs.noaa.gov>.

Base map information shown on this FIRM was provided in digital format by the Commonwealth of Virginia, through the Virginia Geographic Network Division of its Department of Technology Planning (VGIN). These data were produced at scales of 1:2,400 and 1:4,800 from one-foot and two-foot resolution digital orthoregistry flown in Spring 2002.

This map reflects more detailed and up-to-date **stream channel configurations** than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact the **FEMA Map Service Center** at 1-800-358-9616 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study report, and/or digital versions of this map. The FEMA Map Service Center may also be reached by Fax at 1-800-358-9620 and its website at <http://msc.fema.gov>.

If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA-MAP (1-877-336-2627) or visit the FEMA website at <http://www.fema.gov/business/nfp>.



LEGEND

SPECIAL FLOOD HAZARD AREAS SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equalled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AD, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

- ZONE A** No Base Flood Elevations determined.
- ZONE AE** Base Flood Elevations determined.
- ZONE AH** Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
- ZONE AD** Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
- ZONE AR** Special Flood Hazard Area formerly protected from the 1% annual chance flood by a flood control system that was subsequently identified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
- ZONE A99** Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
- ZONE V** Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
- ZONE VE** Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

- ZONE X** Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

- ZONE X** Areas determined to be outside the 0.2% annual chance floodplain.
- ZONE D** Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

- 1% annual chance floodplain boundary
- 0.2% annual chance floodplain boundary
- Floodway boundary
- Zone D boundary
- CBRS and OPA boundary
- Boundary dividing Special Flood Hazard Area Zones and boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.
- Base Flood Elevation line and value; elevation in feet* (EL. 987)
- Base Flood Elevation value where uniform within zone; elevation in feet*

* Referenced to the North American Vertical Datum of 1988

- Cross section line
- Transect line
- Geographic coordinates referenced to the North American Datum of 1983 (NAD 83), Western Hemisphere
- 1000-meter Universal Transverse Mercator grid values, zone 17
- 5000-foot grid ticks: Virginia State Plane coordinate system, South zone (FIPSZONE 4502), Lambert Conformal Conic projection
- Bench mark (see explanation in Notes to Users section of this FIRM panel)
- River Mile

MAP REPOSITORY
Refer to listing of Map Repositories on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP
October 16, 2009

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL

For community map revision history prior to countywide mapping, refer to the Community Map History table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your Insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

MAP SCALE 1" = 2000'

1000 0 2000 4000 FEET
600 0 600 1200 METERS

NATIONAL FLOOD INSURANCE PROGRAM

PANEL 0350D

FIRM
FLOOD INSURANCE RATE MAP

HALIFAX COUNTY, VIRGINIA AND INCORPORATED AREAS

PANEL 350 OF 625
(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:	COMMUNITY	NUMBER	PANEL	SUFFIX
	HALIFAX COUNTY	510188	0350	D

Notice to User: The Map Number shown below should be used when placing map orders; the Community Number shown above should be used on insurance applications for the subject community.

MAP NUMBER
51083C0350D

EFFECTIVE DATE
OCTOBER 16, 2009

Federal Emergency Management Agency

Appendix C: Storm Water Calculations

- Storm Water Run-On Estimate
- Pumping Time Estimate

Storm Water Run-On Estimate



PROJECT / LOCATION: Clover Power Station - Clover, Virginia		PROJECT / PROPOSAL NO.
SUBJECT: Storm Water Capacity of North and South Basins		430492.0000
PREPARED BY: K. Thelen	DATE: 10/7/2021	FINAL <input type="checkbox"/>
CHECKED BY: J. Hotstream	DATE: 10/8/2021	REVISION <input type="checkbox"/>

Purpose: Determine the minimum freeboard necessary to control inflow from a 24-hour, 100-year storm event

Methodology:

- 1.) Determine the volume storm water runoff volume that flows into the basins from the 100 year, 24 hour storm event

$$V_R = \text{Volume of Runoff}$$

$$\text{Area} = 5.78 \text{ ac}$$

$$\text{Rainfall Rate} = 8.01 \text{ in} \quad \text{Design storm data from NOAA, refer to attached sheets}$$

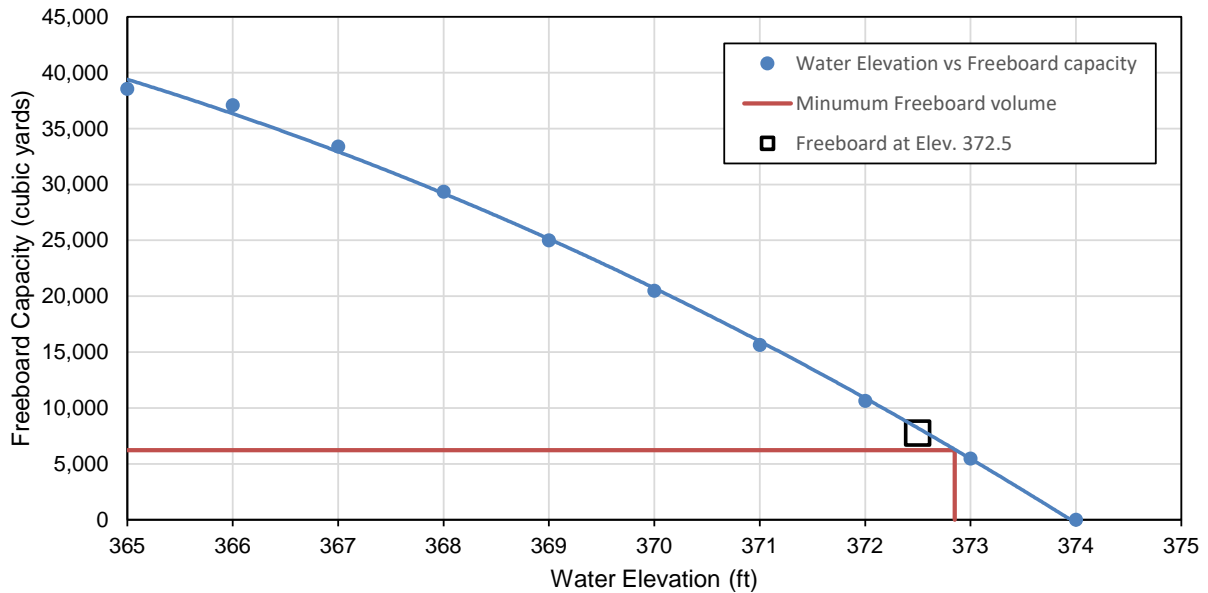
$$V_R = \text{Area} * \text{Rainfall Rate}$$

$$V_R = 6,224 \text{ cy}$$

PROJECT / LOCATION: Clover Power Station - Clover, Virginia		PROJECT / PROPOSAL NO.
SUBJECT: Storm Water Capacity of North and South Basins		430492.0000
PREPARED BY: K. Thelen	DATE: 10/7/2021	FINAL <input type="checkbox"/>
CHECKED BY: J. Hotstream	DATE: 10/8/2021	REVISION <input type="checkbox"/>

- 2.) Find the minimum freeboard elevation which will allow sufficient capacity to control inflow from the 100-year, 24-hour storm event
 - 2a) Plot Freeboard Storage vs Freeboard Elevation
- *plot developed from stage-storage data, refer to attached table

Water Elevation vs Freeboard Capacity



Find minimum Freeboard Elevation such that $S_{FB} > V_R$

$$E_{FB, \min} = 372.85 \text{ ft}$$

- 3.) Choose a conservative Freeboard Elevation, then compare to V_R

$$E_{FB, \text{cons}} = 372.5 \text{ ft}$$

- If freeboard volume (S_{FB}) $> V_R$ then sufficient freeboard is provided

$$S_{FB} = 7,749 \text{ cy at Elevation 372.5 feet}$$

$$V_R = 6,224 \text{ cy}$$

$$S_{FB} > V_R$$

Conclusion: Because the $S_{FB} > V_R$, the 1.5 feet of freeboard is capable of containing the runoff volume of the 100-year, 24-hour storm event



PROJECT / LOCATION: Clover Power Station - Clover, Virginia		PROJECT / PROPOSAL NO.
SUBJECT: Storm Water Capacity of North and South Basins		430492.0000
PREPARED BY: K. Thelen	DATE: 10/7/2021	FINAL <input type="checkbox"/>
CHECKED BY: J. Hotstream	DATE: 10/8/2021	REVISION <input type="checkbox"/>

ATTACHMENTS

From Drawing 6607-15348-CWSG-S3107:

STAGE VS STORAGE MATRIX

ELEV.	North Pond Capacity (CY)	North Pond Capacity (CF)	South Pond Capacity (CY)	South Pond Capacity (CF)	Total Pond Capacity (CY)	Total Pond Capacity (CF)	COMBINED POND CAPACITY (AC FT)	CAPACITY ABOVE TO CREST (AC FT)	CAPACITY ABOVE TO CREST (CY)
365	71	1,924	72	1,937	143	3,861	0.1	23.9	38,561
366	809	21,848	806	21,769	1,615	43,617	1.0	23.0	37,089
367	2,635	71,145	2,613	70,546	5,248	141,691	3.3	20.7	33,457
368	4,700	126,904	4,652	125,617	9,353	252,521	5.8	18.2	29,352
369	6,877	185,666	6,803	183,678	13,679	369,344	8.5	15.5	25,025
370	9,166	247,477	9,066	244,769	18,231	492,246	11.3	12.7	20,473
371	11,570	312,381	11,442	308,930	23,012	621,311	14.3	9.7	15,693
372	14,090	380,420	13,933	376,200	28,023	756,620	17.4	6.6	10,681
373	16,725	451,579	16,539	446,560	33,264	898,138	20.6	3.4	5,440
374	19,461	525,435	19,244	519,583	38,704	1,045,018	24.0	0.0	0



NOAA Atlas 14, Volume 2, Version 3
Location name: Randolph, Virginia, USA*
Latitude: 36.8698°, Longitude: -78.7018°
Elevation: 372.82 ft**
 * source: ESRI Maps
 ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M. Yekta, and D. Riley

NOAA, National Weather Service, Silver Spring, Maryland

[PF_tabular](#) | [PF_graphical](#) | [Maps & aerials](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.364 (0.324-0.408)	0.430 (0.384-0.482)	0.504 (0.449-0.564)	0.564 (0.502-0.629)	0.632 (0.560-0.703)	0.681 (0.600-0.756)	0.727 (0.639-0.808)	0.767 (0.670-0.854)	0.815 (0.708-0.908)	0.852 (0.734-0.950)
10-min	0.582 (0.518-0.652)	0.688 (0.614-0.771)	0.808 (0.720-0.903)	0.903 (0.803-1.01)	1.01 (0.892-1.12)	1.08 (0.956-1.21)	1.16 (1.01-1.28)	1.22 (1.06-1.35)	1.29 (1.12-1.44)	1.34 (1.16-1.50)
15-min	0.727 (0.648-0.815)	0.865 (0.771-0.969)	1.02 (0.910-1.14)	1.14 (1.02-1.27)	1.28 (1.13-1.42)	1.37 (1.21-1.53)	1.46 (1.28-1.62)	1.54 (1.34-1.71)	1.62 (1.41-1.81)	1.68 (1.45-1.88)
30-min	0.997 (0.888-1.12)	1.20 (1.07-1.34)	1.45 (1.29-1.62)	1.65 (1.47-1.85)	1.89 (1.68-2.10)	2.07 (1.82-2.30)	2.24 (1.97-2.49)	2.39 (2.09-2.66)	2.58 (2.24-2.88)	2.73 (2.35-3.04)
60-min	1.24 (1.11-1.39)	1.50 (1.34-1.68)	1.86 (1.66-2.08)	2.15 (1.92-2.40)	2.52 (2.23-2.80)	2.80 (2.47-3.11)	3.08 (2.71-3.42)	3.35 (2.93-3.73)	3.70 (3.22-4.13)	3.98 (3.43-4.44)
2-hr	1.47 (1.31-1.66)	1.77 (1.57-1.99)	2.21 (1.96-2.49)	2.59 (2.29-2.90)	3.07 (2.70-3.43)	3.47 (3.03-3.86)	3.87 (3.35-4.30)	4.27 (3.68-4.75)	4.82 (4.11-5.36)	5.25 (4.45-5.85)
3-hr	1.57 (1.40-1.77)	1.90 (1.69-2.14)	2.37 (2.10-2.67)	2.77 (2.45-3.11)	3.29 (2.90-3.69)	3.72 (3.25-4.16)	4.15 (3.61-4.63)	4.58 (3.96-5.11)	5.17 (4.42-5.77)	5.64 (4.78-6.30)
6-hr	1.92 (1.70-2.17)	2.31 (2.05-2.61)	2.88 (2.55-3.26)	3.39 (2.99-3.82)	4.06 (3.56-4.57)	4.64 (4.04-5.21)	5.23 (4.52-5.87)	5.86 (5.01-6.55)	6.73 (5.68-7.50)	7.45 (6.21-8.31)
12-hr	2.30 (2.07-2.60)	2.77 (2.49-3.13)	3.47 (3.11-3.91)	4.12 (3.67-4.63)	5.00 (4.42-5.59)	5.77 (5.06-6.42)	6.59 (5.72-7.30)	7.46 (6.41-8.26)	8.73 (7.37-9.64)	9.82 (8.16-10.9)
24-hr	2.68 (2.46-2.95)	3.25 (2.98-3.58)	4.15 (3.79-4.57)	4.90 (4.47-5.39)	6.02 (5.44-6.59)	6.97 (6.27-7.63)	8.01 (7.15-8.75)	9.15 (8.10-9.99)	10.8 (9.46-11.8)	12.3 (10.6-13.4)
2-day	3.15 (2.90-3.45)	3.82 (3.52-4.18)	4.84 (4.45-5.29)	5.68 (5.21-6.20)	6.89 (6.29-7.52)	7.91 (7.18-8.63)	9.00 (8.11-9.82)	10.2 (9.10-11.1)	11.9 (10.5-13.0)	13.3 (11.6-14.6)
3-day	3.33 (3.07-3.65)	4.03 (3.72-4.42)	5.11 (4.70-5.59)	6.00 (5.50-6.56)	7.28 (6.64-7.95)	8.34 (7.57-9.10)	9.49 (8.55-10.4)	10.7 (9.59-11.7)	12.5 (11.1-13.7)	14.0 (12.2-15.3)
4-day	3.52 (3.23-3.85)	4.25 (3.91-4.66)	5.39 (4.95-5.89)	6.32 (5.79-6.91)	7.66 (6.99-8.37)	8.78 (7.97-9.57)	9.98 (9.00-10.9)	11.3 (10.1-12.3)	13.1 (11.6-14.4)	14.7 (12.9-16.1)
7-day	4.05 (3.73-4.42)	4.87 (4.49-5.32)	6.07 (5.59-6.62)	7.06 (6.48-7.69)	8.47 (7.73-9.22)	9.64 (8.76-10.5)	10.9 (9.82-11.8)	12.2 (10.9-13.3)	14.1 (12.5-15.4)	15.7 (13.7-17.1)
10-day	4.60 (4.26-5.00)	5.51 (5.10-5.99)	6.80 (6.28-7.38)	7.83 (7.23-8.50)	9.30 (8.55-10.1)	10.5 (9.60-11.4)	11.7 (10.7-12.7)	13.1 (11.8-14.1)	14.9 (13.3-16.2)	16.4 (14.6-17.8)
20-day	6.23 (5.81-6.71)	7.42 (6.92-7.98)	8.96 (8.34-9.63)	10.2 (9.44-10.9)	11.8 (10.9-12.7)	13.1 (12.1-14.1)	14.4 (13.2-15.5)	15.7 (14.4-16.9)	17.5 (15.9-18.9)	18.9 (17.1-20.4)
30-day	7.70 (7.23-8.20)	9.12 (8.57-9.70)	10.8 (10.1-11.5)	12.0 (11.3-12.8)	13.7 (12.8-14.5)	14.9 (13.9-15.8)	16.1 (15.0-17.1)	17.3 (16.1-18.4)	18.9 (17.4-20.1)	20.0 (18.4-21.4)
45-day	9.71 (9.16-10.3)	11.5 (10.8-12.1)	13.4 (12.6-14.2)	14.8 (13.9-15.7)	16.7 (15.6-17.6)	18.0 (16.9-19.1)	19.3 (18.1-20.5)	20.6 (19.2-21.9)	22.2 (20.6-23.6)	23.4 (21.6-24.9)
60-day	11.6 (11.0-12.2)	13.6 (12.9-14.4)	15.7 (14.8-16.6)	17.3 (16.3-18.2)	19.3 (18.2-20.3)	20.7 (19.5-21.9)	22.1 (20.8-23.4)	23.4 (21.9-24.8)	25.1 (23.4-26.6)	26.2 (24.4-27.9)

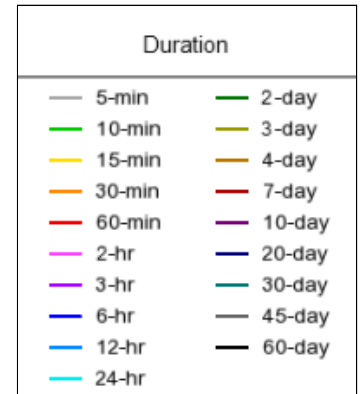
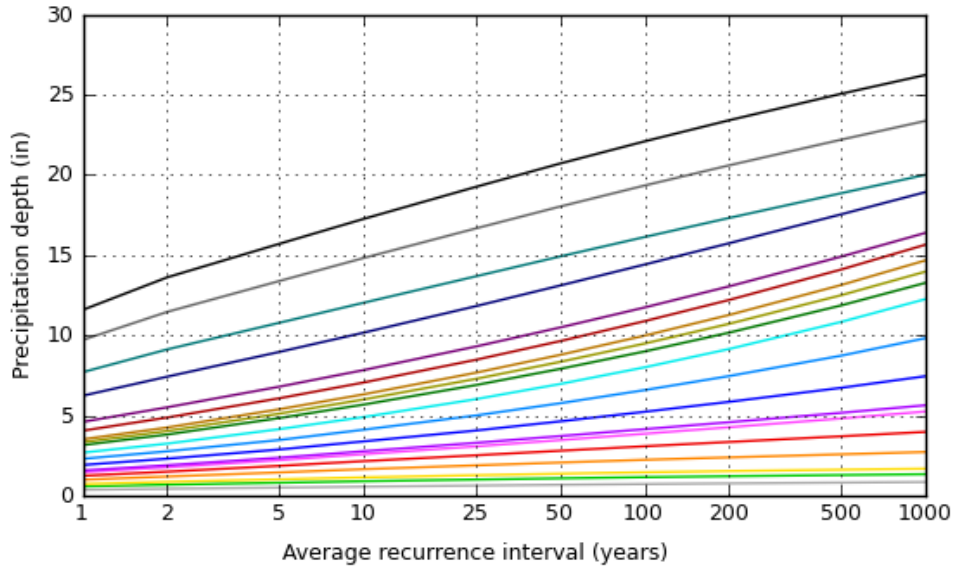
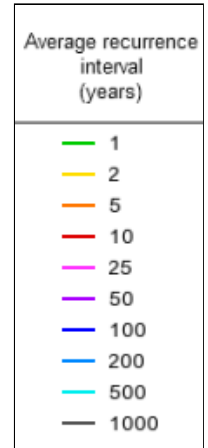
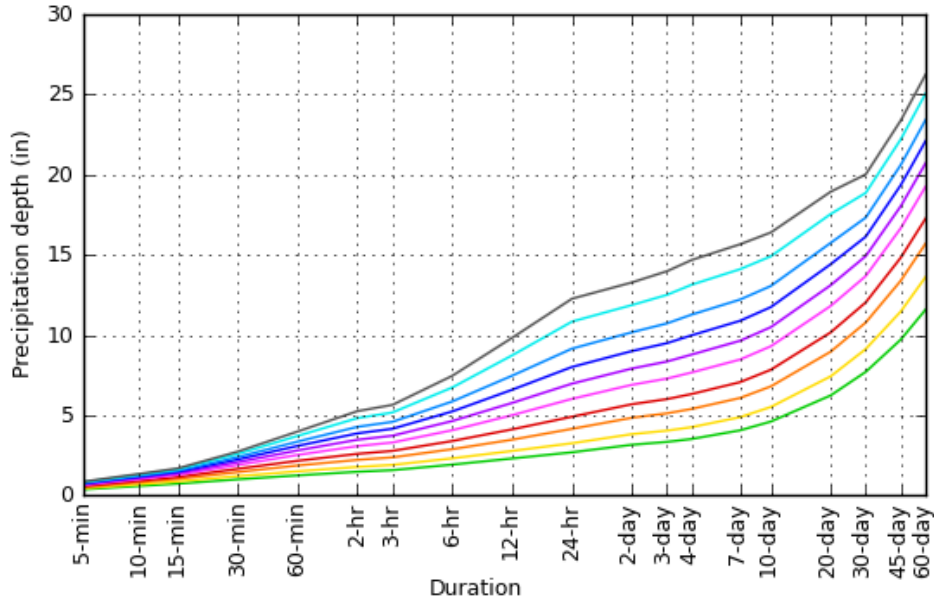
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical

PDS-based depth-duration-frequency (DDF) curves

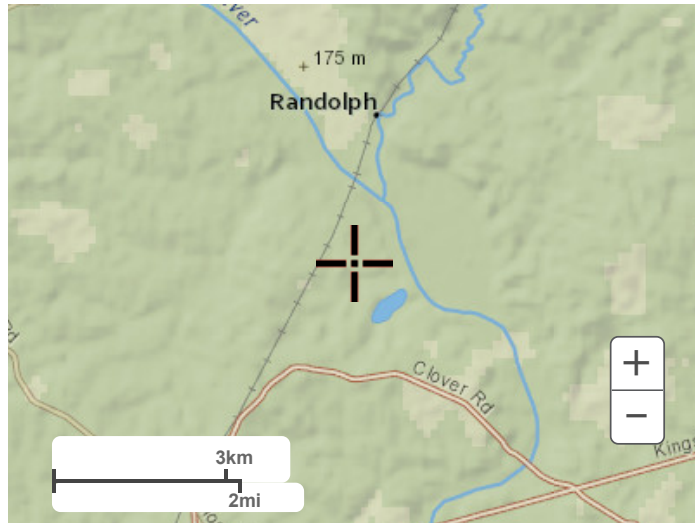
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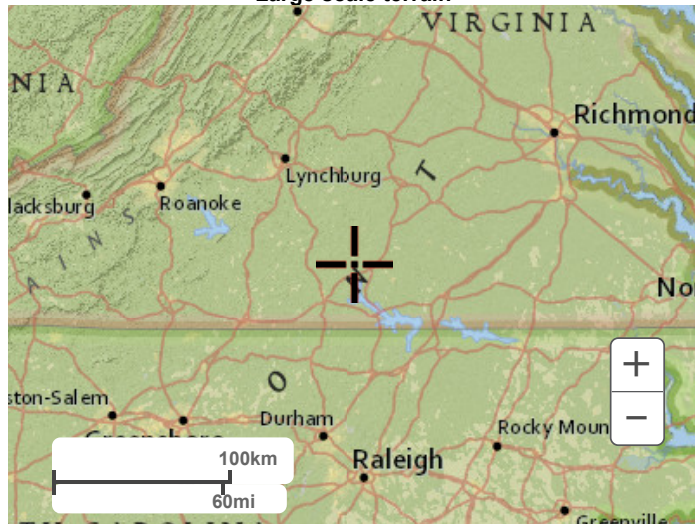
[Back to Top](#)

Maps & arials

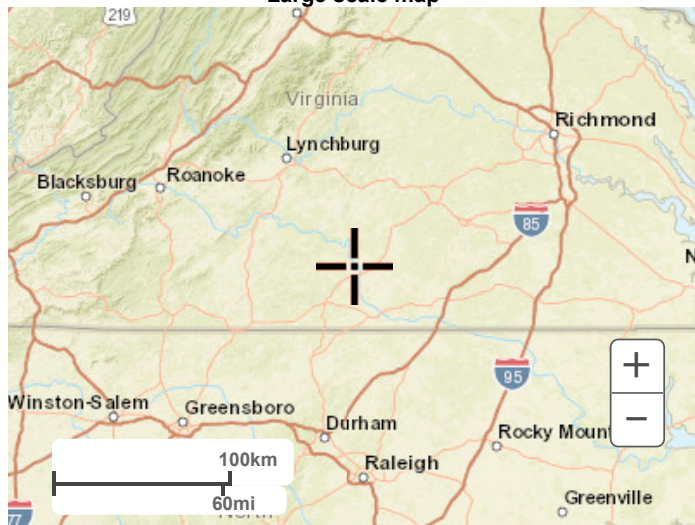
Small scale terrain



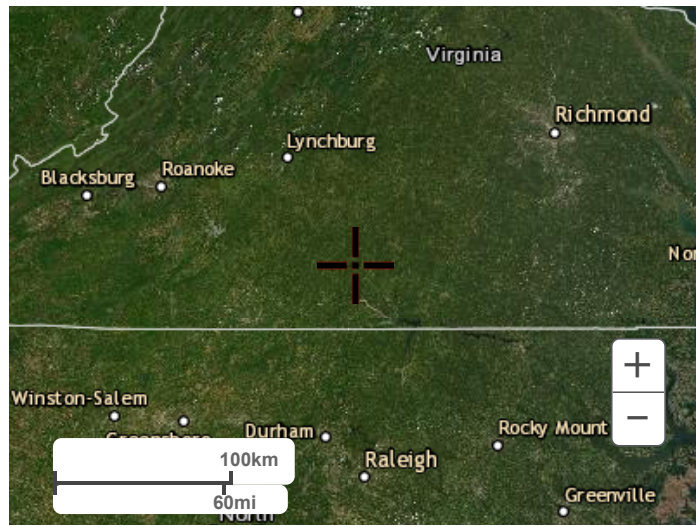
Large scale terrain



Large scale map



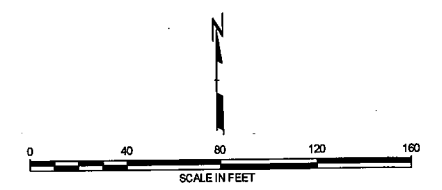
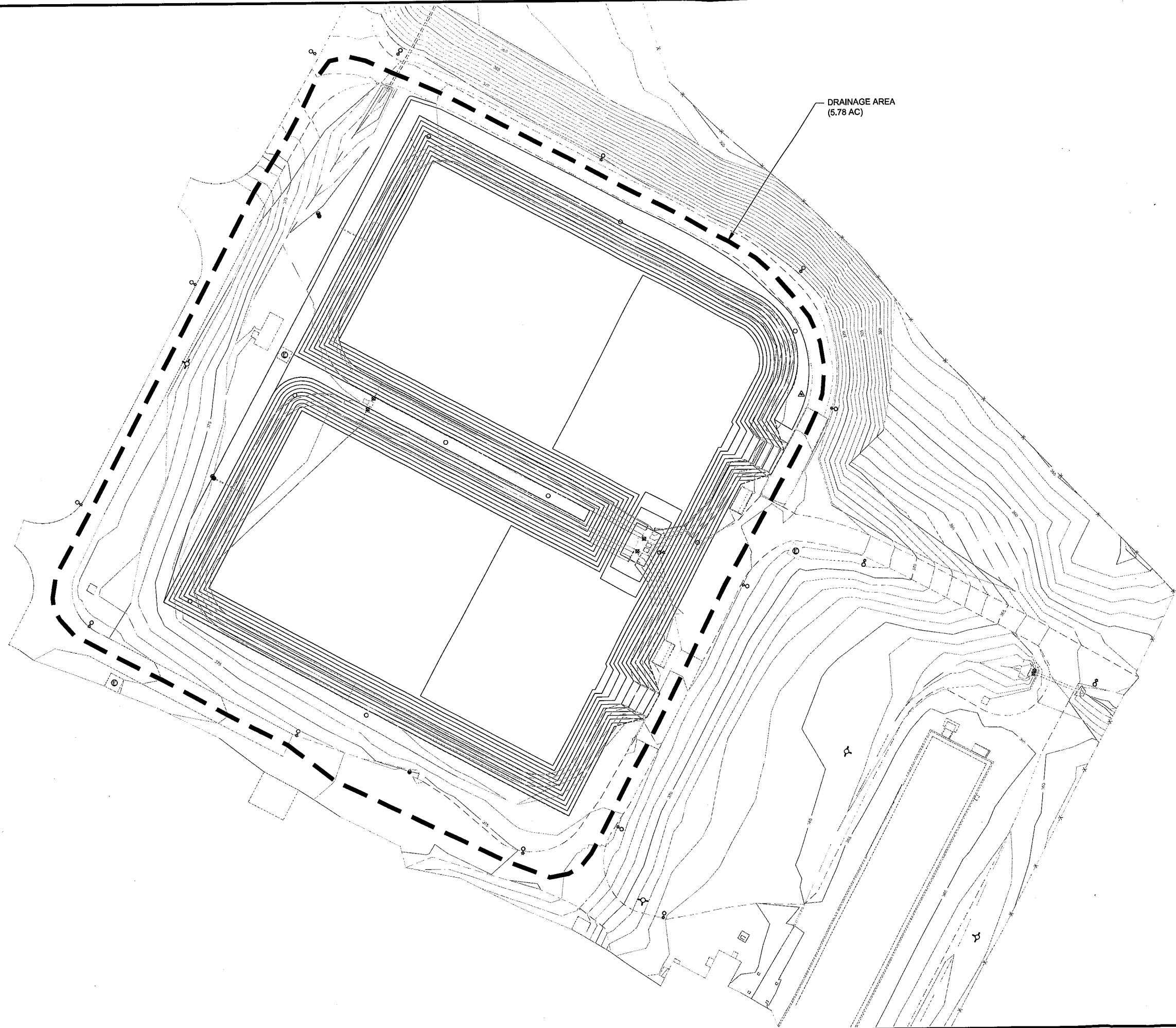
Large scale aerial



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[US Department of Commerce](#)
[National Oceanic and Atmospheric Administration](#)
[National Weather Service](#)
[National Water Center](#)
1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

[Disclaimer](#)



NO.	BY	DATE	REVISION	APPD.
PROJECT: DOMINION RESOURCES SERVICES, INC. CLOVER POWER STATION RETROFIT - EMERGENCY SLUDGE STORAGE PONDS CLOVER, VIRGINIA				
TITLE: WORKING COPY STORMWATER DRAINAGE AREA				
DRAWN BY: E. OLSON		PROJ NO: 232002.0000.0000.000004		
CHECKED BY: --		F-01		
APPROVED BY: --		DATE: NOVEMBER 2015		
		30 Patewood Drive Patewood Plaza One, Suite 300 Greenville, SC 29615 Phone: 864.281.0030		
FILE NO:		F-01_Drainage Areas and Tc.dwg		

06

Plot - Attachment: 11/18/2015 11:11:11 AM - Attachment: 11/18/2015 11:11:11 AM - ELOT DATE: November 18, 2015 - 2:17 PM - LAYOUT: F-01
 DRAWING NAME: P:\V\00000000\Stormwater\F-01_Drainage Areas and Tc.dwg

Pumping Time Estimate



PROJECT / LOCATION: Clover Power Station - Clover, Virginia		PROJECT / PROPOSAL NO.
SUBJECT: Estimated Pump Down Time		430492.0000
PREPARED BY: K. Thelen	DATE: 7/29/21	FINAL <input type="checkbox"/>
CHECKED BY: J. Hotstream	DATE: 8/22/21	REVISION <input type="checkbox"/>

Purpose: Determine the amount of time needed for a Lawrence VPL3200 Pump to remove the storm water collected during the 100-year, 24-hour storm event to design operation elevation (ELE 372)

Methodology:

- 1.) Use the volume of runoff collected in the basins for the 100-year, 24-hour storm (V_R) from the Freeboard Volume Calculation

- Volume of Runoff

$$V_R = 6,224 \text{ cy}$$

- 2.) Use pump capacity rating to determine amount of time to lower the water level in both basins to ELE 372

-Assume only one pump is operational for both ponds

$$\text{Pump Rate} = 410 \text{ gallons per minute (gpm)}$$

$$V_R = 6,224 \text{ cy}$$

$$V_R = 1,257,086 \text{ gal}$$

$$\text{Time} = \frac{V_R}{\text{Pump Rate}}$$

$$\text{Time} = 3,066 \text{ min}$$

$$= 51.1 \text{ hr}$$

$$= 2.1 \text{ days}$$

Conclusion: It will take approximately 2 days to pump out the storm water to reestablish freeboard after the 100-year, 24-hour storm event.

This calculation assumes that only one pump will be in operation to remove stormwater.