

Ash Pond Inflow Design Flood Control System Plan

Wateree Station Richland County, South Carolina

October 2021

Jonathan Hotstream Senior Scientist

Prepared For:

Dominion Energy of South Carolina 142 Wateree Station Road Eastover, South Carolina 29044

Prepared By:

TRC

50 International Drive, Suite 150 Greenville, South Carolina 29615

Nakia Addison, P.E. Office Practice Leader





TABLE OF CONTENTS

| 1.0 | BACKGROUND | 1 |
|-----|--------------------------------------|-----|
| | 1.1 Existing Conditions | . 1 |
| 2.0 | FEDERAL REGULATIONS | 2 |
| 3.0 | FLOW INTO THE ASH POND | 3 |
| 4.0 | FLOW OUT OF THE ASH POND | 4 |
| 5.0 | CONCLUSION | 5 |
| 6.0 | AMENDMENT AND PERIODIC PLAN REVISION | 6 |
| 7.0 | REFERENCES | 7 |
| 8.0 | CERTIFICATION | 8 |

FIGURES

Figure 1: Site Location Map Figure 2: Site Overview Map

APPENDICES

Appendix A: Select Engineering Drawings Appendix B: Flood Insurance Rate Map Appendix C: Stormwater Calculations



1.0 Background

Dominion Energy of South Carolina (DESC) owns and operates the Wateree Station (Station). The purpose of this Inflow Design Flood Control System Plan (Plan) is to present the flood control features of the Ash Pond at the Station as required by the United States Environmental Protection Agency's (USEPA) final coal combustion residuals (CCR) rule Title 40 Code of Federal Regulations (40 CFR) Part 257 Subpart D - "Standards for the Disposal of Coal Combustion Residuals in Landfills and Surface Impoundments." The requirements for Inflow Design Flood Control System Plans for surface impoundments are presented in 40 CFR 257.82. The initial Inflow Design Flood Control System Plan was developed and placed in the Station's operating record on October 17, 2016. A periodic revision to the Plan is required every 5 years pursuant to 40 CFR 257.82(c)(4).

This Plan is a periodic revision based on review of the initial Plan, the recent closure activities that have occurred at the Ash Pond, and a site visit by TRC to observe existing conditions.

1.1 Existing Conditions

The Station is located on the Wateree River in Richland County, South Carolina, near the Town of Eastover, refer to Figure 1. The Ash Pond was an approximately 80-acre surface impoundment that was used for the management of sluiced fly and bottom ash material, as well as other low volume plant wastewaters. A Closure Plan was prepared for the Ash Pond in 2013. The Closure Plan was approved by the South Carolina Department of Health and Environmental Control (SC DHEC) in February 2013 (LOA-004214).

Closure activities began in the northern portion of the Ash Pond (Phase 1) in July 2016, and the southern portion of the Ash Pond (Phase 2) in July 2017. Closure activities included the removal of CCR as well as a minimum two feet of residual soils under the CCR. Upon removal of the CCR and soil, the Ash Pond area was backfilled with clean, native soils to a grade that would prevent ponding and enable stormwater to gravity discharge from the Ash Pond area via stormwater Outfall SW-003 and NPDES Outfall 03B (Garrett & Moore, 2018, 2019). Phase 1 of the Ash Pond closure construction was completed in November 2017, and Phase 2 construction was completed in October 2019. Closure Certification Reports were prepared by Garrett & Moore in February 2018 and October 2019. Refer to Appendix A for final grades record drawings for Phase 1 and Phase 2 closures.

A closeout notification of successful completion of Phase 1 closure was provided by SC DHEC in March 2018. Closeout notification of successful completion of Phase 2 closure was provided by SC DHEC in October 2019.

Effluent discharge from the Ash Pond is regulated under a National Pollution Discharge Elimination System Permit. Operational and groundwater requirements continue for the pond under the CCR rule.



2.0 Federal Regulations

An owner or operator of an existing or new CCR surface impoundment or any lateral expansion must design, construct, operate, and maintain an inflow design flood control system for the CCR surface impoundment to: (1) adequately manage flow into the CCR surface impoundment during and following the peak discharge of the inflow design flood, and (2) adequately manage flow from the CCR unit to collect and control the peak discharge resulting from the inflow design flood. The inflow design flood is based on the hazard potential classification of the unit as required by 40 CFR 257.73. The inflow design floods for specific hazard potential classifications are as follows:

- (1) The probable maximum flood for high hazard potential CCR surface impoundments
- (2) The 1,000-year flood for significant hazard potential CCR surface impoundments
- (3) The 100-year flood for low hazard potential CCR surface impoundments
- (4) The 25-year flood for incised CCR surface impoundments

The former Ash Pond at the Wateree Station was classified as an incised CCR surface impoundment (Garrett & Moore, 2016): therefore, the Ash Pond must be designed and constructed to adequately manage peak discharges from a 25-year, 24-hour flood. Since the original classification, the Pond's CCR material has been removed and graded so that CCR and liquids are not impounded within the area.



3.0 Flow into the Ash Pond

The Ash Pond was constructed in an area beyond the 100-year floodplain, refer to Appendix B.

The Ash Pond receives no flows from the Station. Removal of CCR, over-excavation of soils, and final grading was competed in October 2019. Therefore, inflow into the former Ash Pond is limited to stormwater that falls within the former Ash Pond footprint and drainage area. During closure of the Ash Pond, the area was graded to provide gentle slopes for overland flow to swales located at the perimeter of the former Ash Pond footprint. These swales convey stormwater to Outfall SW-003 and NPDES Outfall 03B. The outlets are open channel features that do not impound stormwater within the Ash Pond area.

The stormwater modeling software HydroCAD, was utilized to estimate stormwater volume, peak flow rates, and associated velocities for the design 24-hour, 25-year storm event. HydroCAD is largely based on the United States Department of Agriculture Soil Conservation Service's, Technical Release 55 (TR-55) and TR-20 hydrology methods.

TRC performed stormwater analysis considering the following:

- The precipitation volume for the 24-hour, 25-year storm event is 6.66 inches.
- The drainage area and swale geometry are based on the final grades record drawings in Appendix A.

Based on the HydroCAD results (Appendix C) stormwater collected within the footprint of the former Ash Pond during the 24-hour, 25-year storm is adequately managed by the constructed features.

During the site visit the former Ash Pond area had an established vegetative cover with no distressed areas and no erosion or ponding was observed. The site observations confirm the results of the stormwater modeling.



4.0 Flow out of the Ash Pond

During the Ash Pond closure construction, the site was graded to not allow water to be impounded within the former Ash Pond. Closure construction removed CCR therefore stormwater does not come in contact with CCR.

Discharge from the Ash Pond is regulated in accordance with a National Pollutant Discharge Elimination System (NPDES) permit issued by the SC DHEC. The permit grants the Station permission to discharge from the facility to the Wateree River in accordance with effluent limitations, monitoring requirements, and other conditions. The NPDES permit is issued in accordance with the provisions of the Federal Clean Water Act. Therefore, by complying with the NPDES permit, the discharge from the former Ash Pond is also being handled in accordance with the applicable surface water requirements.

Stormwater that enters into the former Ash Pond sheet flows and discharges to either stormwater Outfall SW-003 and NPDES Outfall 03B via engineered ditches. The northern area of the Ash Pond, closed during Phase 1 of the closure, drains to Outfall SW-003. Outfall SW-003 is an open channel swale that discharges to the Wateree River. This swale has a fabric formed concrete lining to protect against erosion. Stormwater that contacts the surface of the northern area closure flows to two perimeter ditches located either along the northern perimeter or eastern perimeter of the former Ash Pond area. These perimeter ditches are lined with permanent turf reinforcement mat, LANDLOK 450 which has allowable flow rates of up to 18 feet per second.

The southern area of the Ash Pond drains to NPDES Outfall 03B, which is an 8-foot-wide box culvert that discharges to the Wateree River. The discharge from the box culvert is stabilized with riprap to prevent erosion. Stormwater runoff from the southern area cover flows to a perimeter drainage ditch along the eastern side of the Ash Pond area that routes the stormwater to Outfall 03B.

Stormwater Flow Summary:

- Northern Area (Phase 1 of Closure):
 - Perimeter Ditch 1:
 - Maximum Peak Flow from 24-hour, 25-year storm: 37 cubic feet per second (cfs);
 velocity: 6 feet per second (ft/sec)
 - Flow Capacity: 364 cfs; allowable velocity: 18 ft/s
- Southern Area (Phase 2 of Closure):
 - Perimeter Ditch:
 - Maximum Peak Flow from 24-hour, 25-year storm: 122 cfs, velocity: 3 ft/sec
 - Flow Capacity: 110 cfs; allowable velocity: 5 ft/s

Based on the stormwater calculations in Appendix C and as-built documentation, the discharge of the 25-year, 24-hour storm is managed in a controlled manner that does not result in erosive flows.



5.0 Conclusion

The Wateree Station Ash Pond meets the requirements of 40 CFR 257.82 of adequately controlling the inflows and outflows of peak discharge for the following reasons:

- The Ash Pond is constructed outside of the 100-year floodplain.
- Ash Pond removal of CCR, over-excavation of soils, and final grading was achieved in 2019. Construction included final grading to prevent ponding of stormwater in the former Ash Pond area.
- The Ash Pond can adequately receive and manage the discharges of direct rainfall from a 25-year, 24-hour storm. The Ash Pond was graded, during closure, to prevent water from being impounded. Stormwater that enters the former Ash Pond footprint discharges via stormwater Outfall SW-003 and NPDES Outfall 03B.
- The discharge from the Ash Pond is permitted under a NPDES permit which was issued in accordance with the provisions of the Federal Clean Water Act.



6.0 Amendment and Periodic Plan Revision

This plan has been completed in compliance with the requirements set forth in 40 CFR 257.82. This document will be placed in the Station's CCR operating record, posted to the publicly accessible CCR website, and government notifications will be provided.

A periodic inflow design flood control system plan must be prepared every 5 years from the completion date of this Plan. However, because the Ash Pond is in the process of confirming its closure, this Plan revision is anticipated to be the last. If closure for the Ash Pond is not confirmed, the next Plan revision is required by October 2026.

The Plan must be amended whenever the periodic review period is reached, or if changes in site conditions occur that will substantially affect the current written Plan.



7.0 References

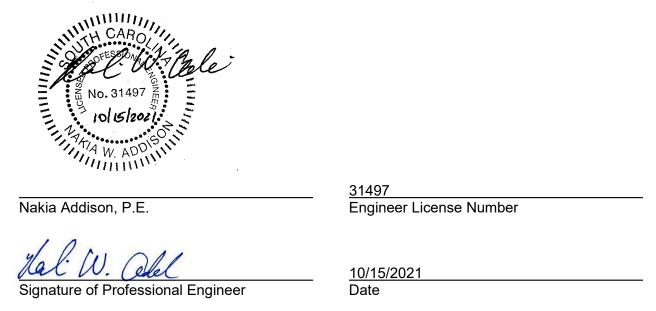
- National Flood Insurance Program. 2017. Flood Insurance Rate Map: Richland County, South Carolina Panel 560 of 650. Map Number 45079C0560L. Map Revised December 21, 2017. Federal Emergency Management Agency. Washington, D.C.
- Garrett & Moore. 2016. Inflow Design Flood Control System Plan for the Wateree Station Ash Pond. Richland County, South Carolina. July 2016.
- Garrett & Moore. 2018. Closure Certification Report for Wateree Station Ash Pond 1 Phase 1. Richland County, South Carolina. February 2018.
- Garrett & Moore. 2019. Closure Certification Report for Wateree Station Ash Pond Phase 2. Richland County, South Carolina. October 2019.



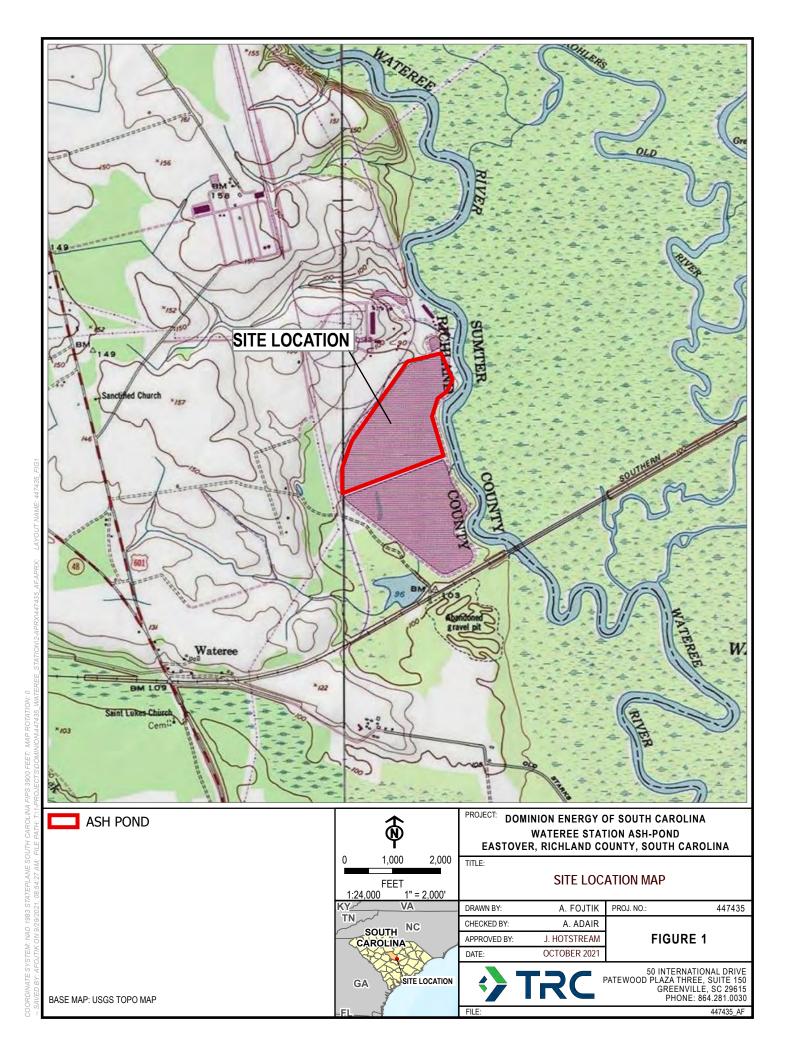
8.0 Certification

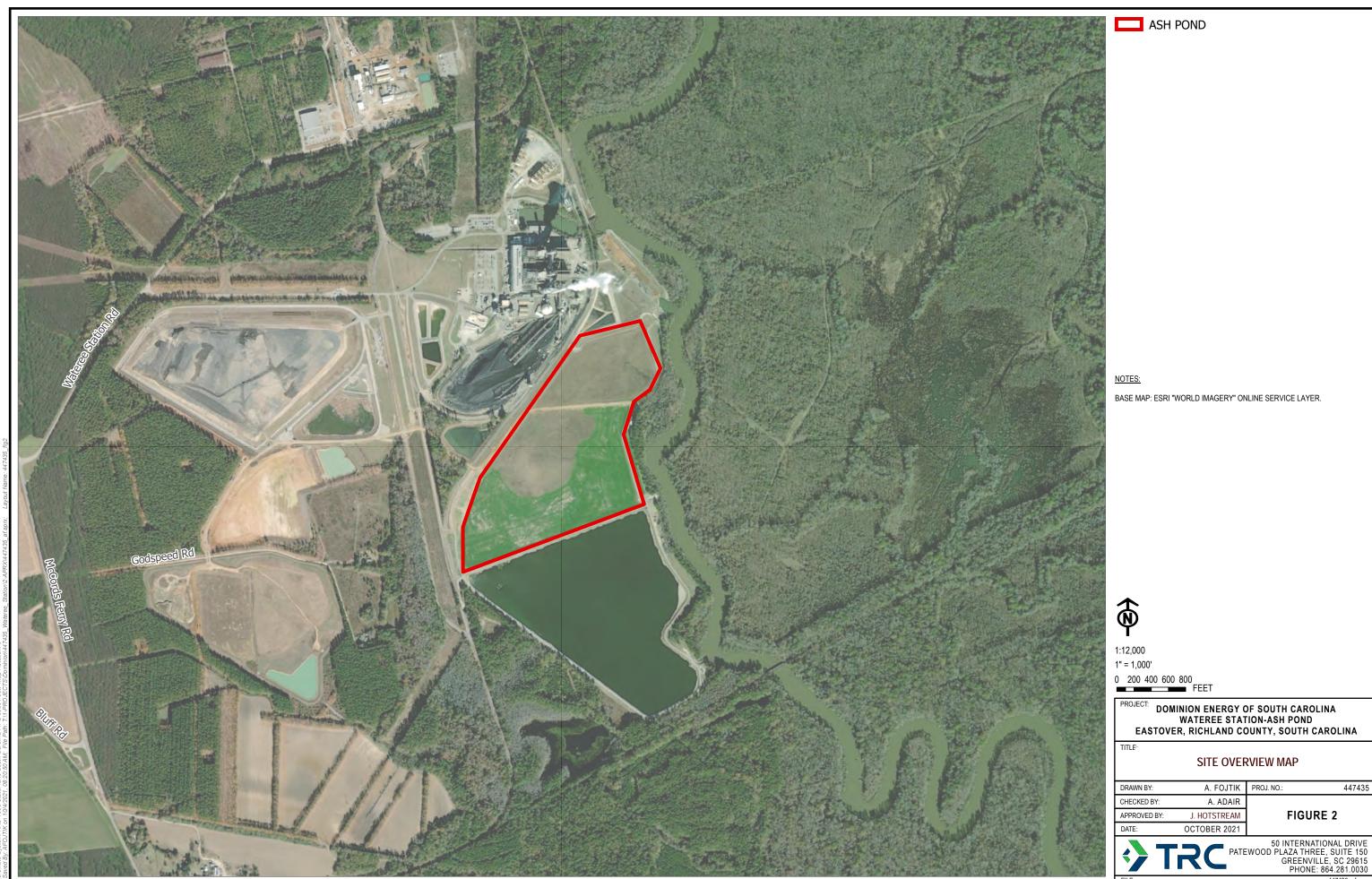
I, the undersigned South Carolina Professional Engineer, hereby certify that I am familiar with the technical requirements of 40 CFR 257 Subpart D. I also certify that it is my professional opinion that, to the best of my knowledge, information, and belief, that the information in this demonstration is in accordance with current good and accepted engineering practice(s) and standard(s) and meets the requirements of paragraph (c) in 40 CFR 257.82.

For the purpose of this document, "certify" and "certification" shall be interpreted and construed to be a "statement of professional opinion." The certification is understood and intended to be an expression of my professional opinion as a South Carolina Licensed Professional Engineer, based upon knowledge, information, and belief. The statement(s) of professional opinion are not and shall not be interpreted or construed to be a guarantee or a warranty of the analysis herein.



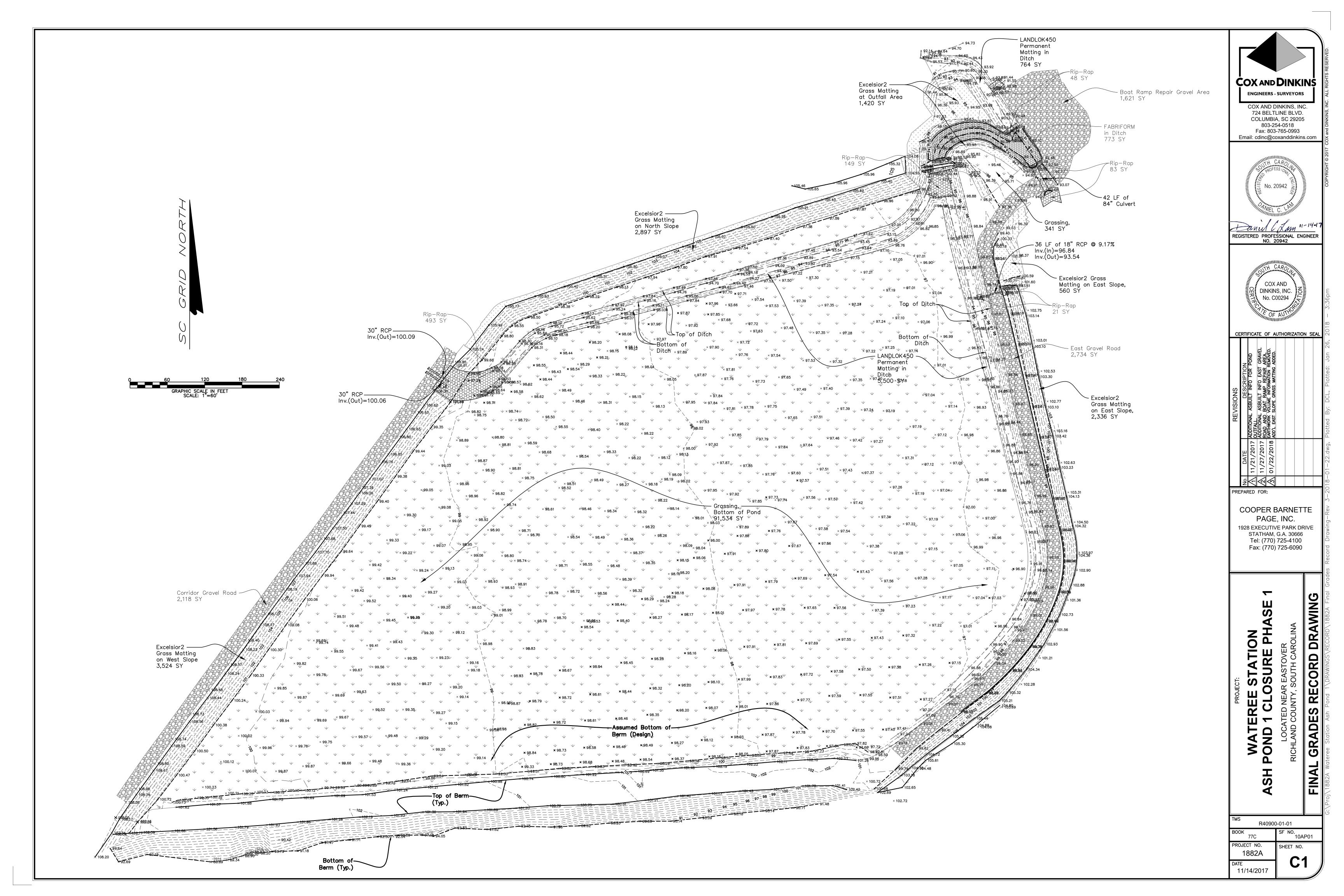


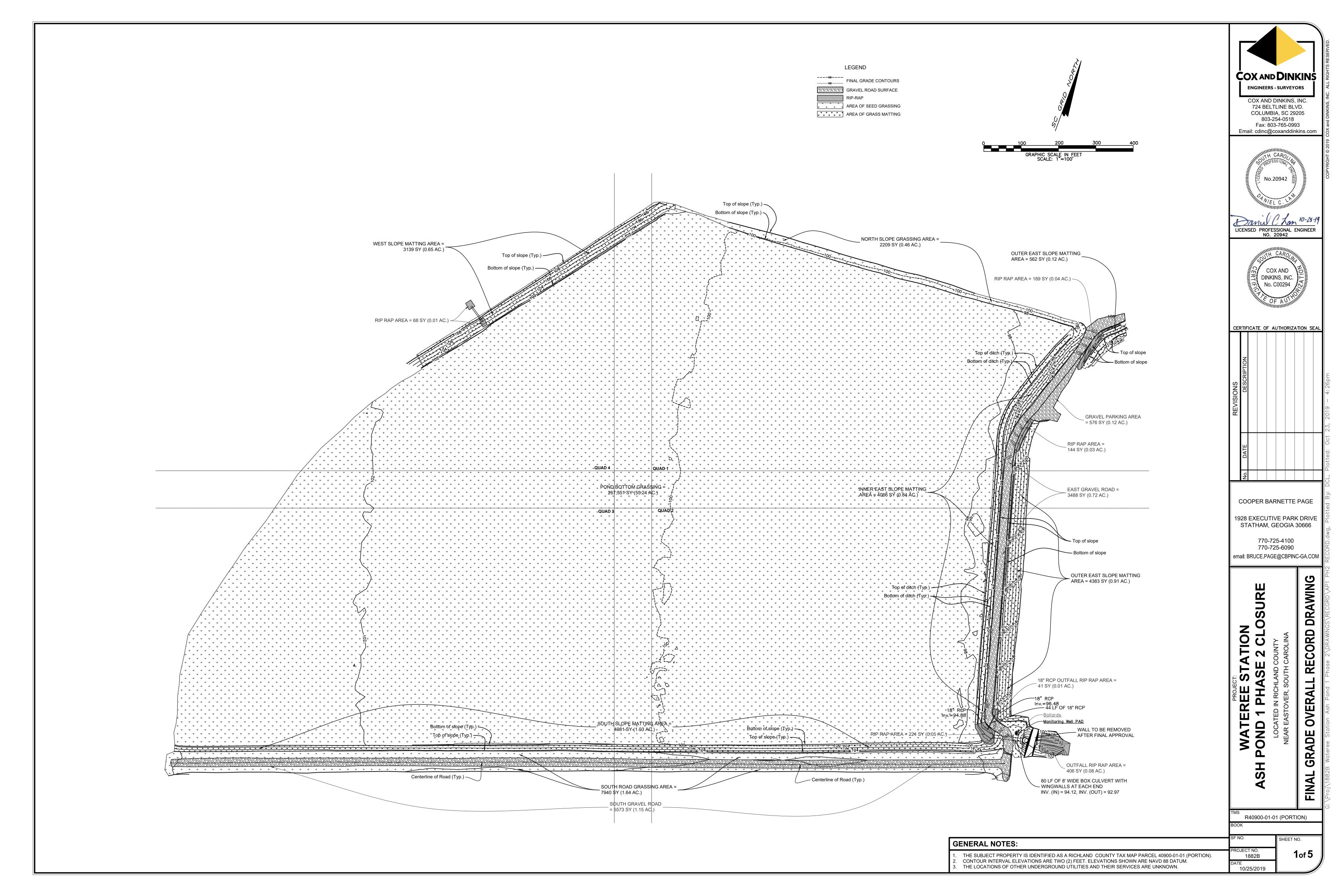






Appendix A: Select Engineering Drawings







Appendix B: Flood Insurance Rate Map

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in sreas where Base Flood Elevations (BFEs) and/or floodways have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Sithwater Elevations tables contained within the Flood Insurance Study (FlS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent counted whole-doctors. These BFEs are intended for need insurance rating purposes only and relevations. These BFEs are intended for need insurance rating purposes only and the FIRM of the FIRM and the FIRM of the FIRM of purposes of construction and/or floodplain management.

Coastal Base Flood Elevations (BFEs) shown on this map apply only landward of 0.0" North American Vertical Datum of 1988 (NAVID 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Sillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Sillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Porgarm. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Lambert Conformal Conic State Plane South Carolina FIRS 3900. The horizontal datum was NAD83, GRS1980 shorehol. Differences in datum, spheroid, projection or State Plane zones used in the production of FIRMs for adjacent jurisdictions may result in slight procitional differences in map features across jurisdiction boundaries. These differences on rot affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These food elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1982 and the North American Vertical Datum of 1989, sust the National Geodetic Survey website at https://www.cigs.coaa.gov/ or contact the National Geodetic Survey at the following address:

NUJAA, NVNGS12 National Geodetic Survey SSMC-3, #9202 1315 East-West Highway Silver Spring, Maryland 20910-3282 (301) 713-3242

shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at http://www.ngs.noaa.gov/.

Base map information shown on this FIRM was provided in digital format by Richland County, South Carolina.

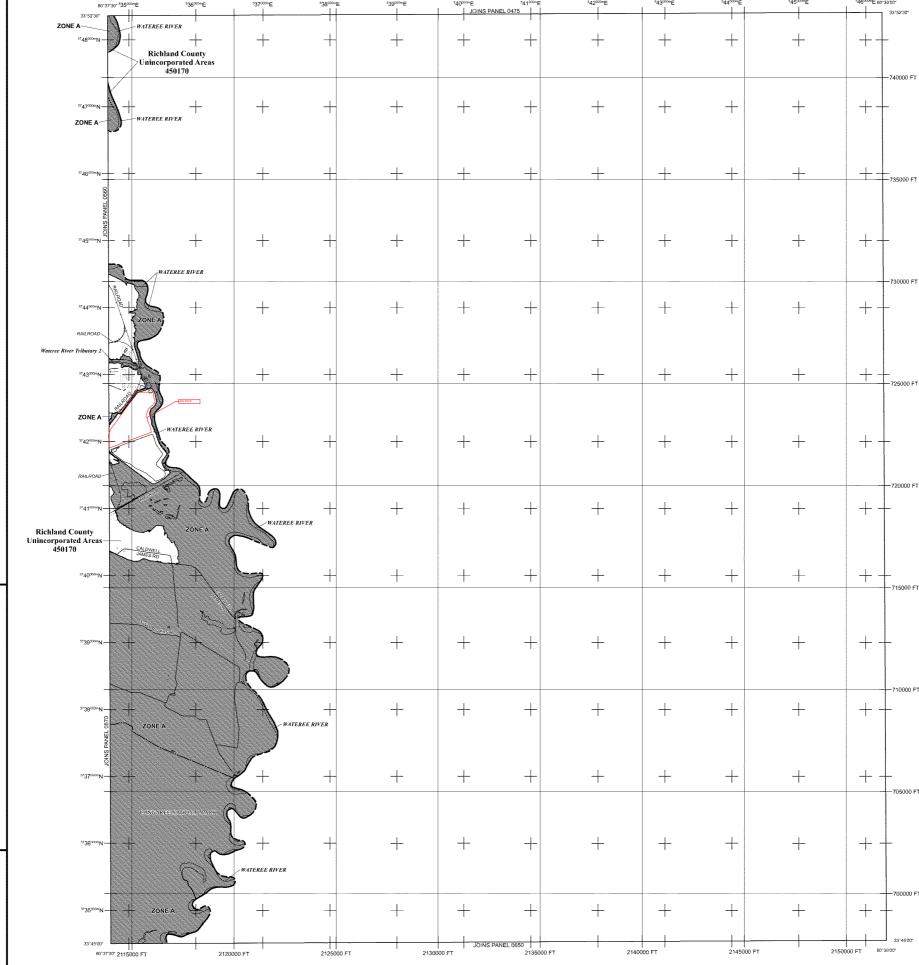
This map reflects more detailed and up-to-date stream channel configurations than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

For information and questions about this map, available products associated with this FIRM including historic versions of this FIRM, how to order products or the National Flood Insurance Program in general, please call the FEMA Mapping Information eXchange at 1-877-EMA-MAP (1-877-336-2627) or visit the FEMA Map Service weekslie at http://www.msc.fema.gov/. Available products may include previously issued Letters of Map Change, a Flood Insurance Report, and/or digital versions of this map. Mary of these products can be ordered or obtained directly from the website. Users may determine the current map date for each FIRM panel by visiting the FEMA Map Service Center website or by calling the FEMA Map Information eXchange.

The profile base lines depicted on this map represent the hydraulic modeling baselines that match the flood profiles in the FIS report. As a result of improved topographic data, the profile base line, in some cases, may deviate significantly from the channel centerline or appear outside the SFHA.



LEGEND

SPECIAL FLOOD HAZARD AREAS SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual knowner floot) also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard kneal is the area subject to flooding by the 1% annual chance flood. Anses of Special Flood Hazard roke as the change of the special Flood Hazard roked 2 Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

No Base Flood Elevations determined.

Base Flood Elevations determined.

Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.

Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.

Special Flood Hazard Area formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood briefly.

OTHER FLOOD AREAS

Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 floot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

III

ZONE X

 $\langle A \rangle$

23)-

ZONE X Areas determined to be outside the 0.2% annual chance floodplain Areas in which flood hazards are undetermined, but possible. ZONE D

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

Floodway boundary

Zone D boundary

~~~ 513 ~~~ Base Flood Elevation line and value; elevation in feet\* Base Flood Elevation value where uniform within zone; elevation in feet\* (EL 987)

Referenced to the North Ar tical Datum of 1988 **⊸**A Cross section line

-23

Geographic coordinates referenced to the North American Datum of 1983 (NAD 83), Western Hemisphere 97"07"30", 32"22"30" 4275000mp 1000-meter Universal Transverse Mercator orld ticks, zone 17

1000-freet onlinests interverse metatur grid octs, 201e 17 5000-foot grid values: South Carolina State Piane coordinate system (FIPSZONE = 3900), Lambert projection Bench mark (see explanation in Notes to Users section of this FIRM panel) 6000000 FT

MAP REPOSITORIES Refer to Map Repositories List on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP January 19, 1994

MAP SCALE 1" = 2000"

AP SCALE 1" = 2000"

1.000 2.000 3.000 4.000

1.001 1.000 1.000 1.000 1.000 1.000

1.001 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1 1,000 0 1,000

NFIP

Œ

PANEL 0600L FIRM FLOOD INSURANCE RATE MAP

RICHLAND COUNTY, SOUTH CAROLINA AND INCORPORATED AREAS

PANEL 600 OF 650

(SEE MAP INDEX FOR FIRM PANEL LAYOUT) CONTAINS:

450170 0600 L



45079C0600L MAP REVISED **DECEMBER 21, 2017** 

MAP NUMBER

Federal Emergency Management Agency

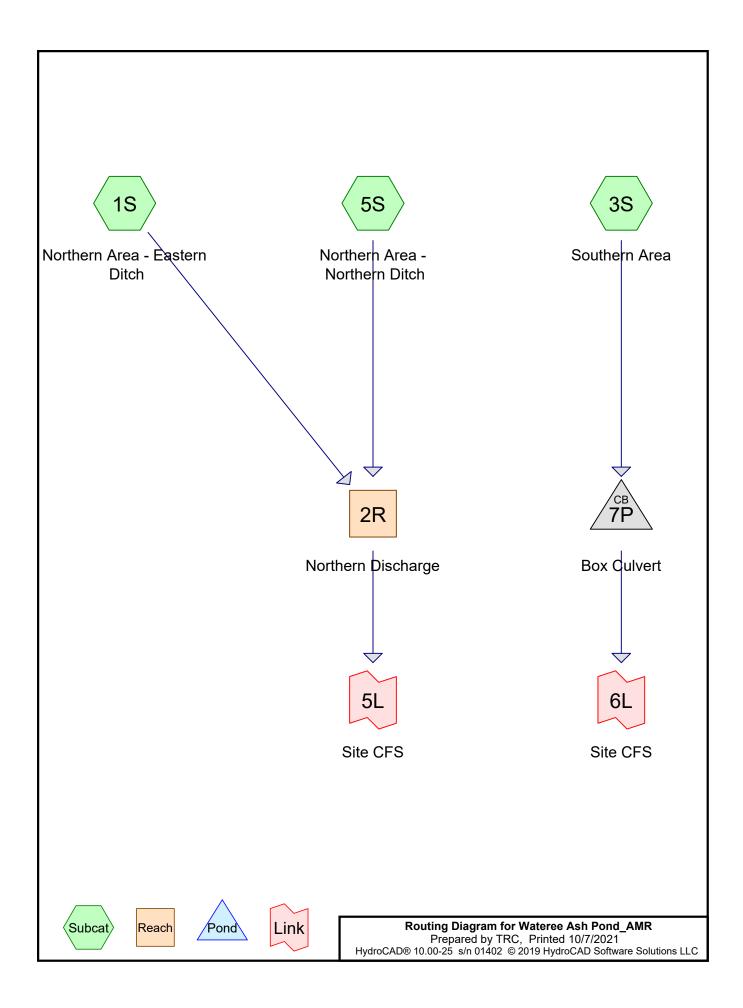
http://www.dnr.state.sc.us/

DNR

This digital Flood Insurance Rate Map (FIRM) was produced through a unique cooperative partnership between the State of South Carolina and the Federal Emergency Management Agency (FEMA). The State of South Carolina has implemented a long term approach of Boodbain management to decrease the costs associated with flooding. This is demonstrated by the State's commitment to map floodplain areas at the local level. As a part of this effort, the state of South Carolina has joined in a Cooperating Technical State agreement with FEMA to produce and maintain this digital FIRM.



## **Appendix C: Stormwater Calculations**



Prepared by TRC
HydroCAD® 10.00-25 s/n 01402 © 2019 HydroCAD Software Solutions LLC

## Page 2

Printed 10/7/2021

## **Area Listing (selected nodes)**

| 8  | 6.379  | 75 | TOTAL AREA                                 |
|----|--------|----|--------------------------------------------|
|    | 2.924  | 96 | Gravel surface, HSG C (1S, 3S, 5S)         |
| 8  | 3.455  | 74 | >75% Grass cover, Good, HSG C (1S, 3S, 5S) |
| (a | icres) |    | (subcatchment-numbers)                     |
|    | Area   | CN | Description                                |

Printed 10/7/2021

Page 3

## Soil Listing (selected nodes)

| Area    | Soil  | Subcatchment      |
|---------|-------|-------------------|
| (acres) | Group | Numbers           |
| 0.000   | HSG A | _                 |
| 0.000   | HSG B |                   |
| 86.379  | HSG C | 1S, 3S, 5S        |
| 0.000   | HSG D |                   |
| 0.000   | Other |                   |
| 86.379  |       | <b>TOTAL AREA</b> |

Prepared by TRC
HydroCAD® 10.00-25 s/n 01402 © 2019 HydroCAD Software Solutions LLC

Printed 10/7/2021 Page 4

## **Ground Covers (selected nodes)**

| HSG-A   | HSG-B   | HSG-C   | HSG-D   | Other   | Total   | Ground                 | Subcatchment |
|---------|---------|---------|---------|---------|---------|------------------------|--------------|
| (acres) | (acres) | (acres) | (acres) | (acres) | (acres) | Cover                  | Numbers      |
| 0.000   | 0.000   | 83.455  | 0.000   | 0.000   | 83.455  | >75% Grass cover, Good | 1S, 3S,      |
|         |         |         |         |         |         |                        | 5S           |
| 0.000   | 0.000   | 2.924   | 0.000   | 0.000   | 2.924   | Gravel surface         | 1S, 3S,      |
|         |         |         |         |         |         |                        | 5S           |
| 0.000   | 0.000   | 86.379  | 0.000   | 0.000   | 86.379  | TOTAL AREA             |              |

Prepared by TRC
HydroCAD® 10.00-25 s/n 01402 © 2019 HydroCAD Software Solutions LLC

Printed 10/7/2021

Page 5

## Pipe Listing (selected nodes)

| Line# | Node   | In-Invert | Out-Invert | Length | Slope   | n     | Diam/Width | Height   | Inside-Fill |
|-------|--------|-----------|------------|--------|---------|-------|------------|----------|-------------|
|       | Number | (feet)    | (feet)     | (feet) | (ft/ft) |       | (inches)   | (inches) | (inches)    |
| 1     | 7P     | 94.12     | 92.97      | 80.0   | 0.0144  | 0.017 | 96.0       | 96.0     | 0.0         |

Type II 24-hr 25-yr, 24-hr Rainfall=6.66"

Prepared by TRC Printed 10/7/2021

HydroCAD® 10.00-25 s/n 01402 © 2019 HydroCAD Software Solutions LLC

Page 6

Time span=5.00-48.00 hrs, dt=0.03 hrs, 1434 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment1S: Northern Area - Eastern** Runoff Area=21.160 ac 0.00% Impervious Runoff Depth=3.85" Flow Length=2,237' Tc=74.9 min CN=75 Runoff=36.91 cfs 6.791 af

**Subcatchment3S: Southern Area**Runoff Area=62.759 ac 0.00% Impervious Runoff Depth=3.85"

Flow Length=2,062' Tc=64.9 min CN=75 Runoff=121.80 cfs 20.142 af

**Subcatchment5S: Northern Area - Northern** Runoff Area=2.460 ac 0.00% Impervious Runoff Depth=3.85" Flow Length=906' Tc=4.6 min CN=75 Runoff=17.47 cfs 0.790 af

**Reach 2R: Northern Discharge** Avg. Flow Depth=1.30' Max Vel=2.45 fps Inflow=37.91 cfs 7.581 af n=0.069 L=200.3' S=0.0144 '/' Capacity=787.13 cfs Outflow=37.88 cfs 7.581 af

Pond 7P: Box Culvert Peak Elev=97.19' Inflow=121.80 cfs 20.142 af 96.0" x 96.0" Box Culvert n=0.017 L=80.0' S=0.0144 '/' Outflow=121.80 cfs 20.142 af

**Link 5L: Site CFS**Inflow=37.88 cfs 7.581 af
Primary=37.88 cfs 7.581 af

**Link 6L: Site CFS**Inflow=121.80 cfs 20.142 af
Primary=121.80 cfs 20.142 af

Total Runoff Area = 86.379 ac Runoff Volume = 27.723 af Average Runoff Depth = 3.85" 100.00% Pervious = 86.379 ac 0.00% Impervious = 0.000 ac

Page 7

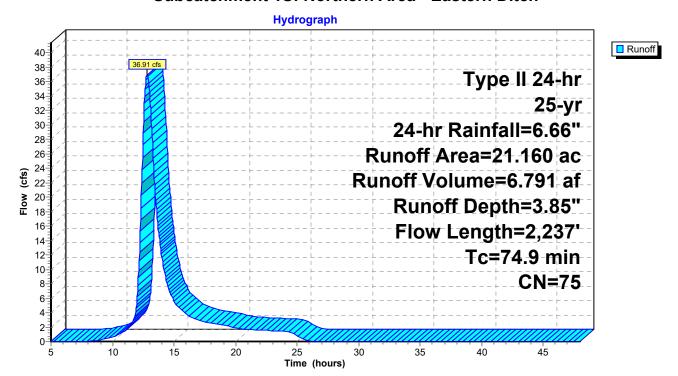
## **Summary for Subcatchment 1S: Northern Area - Eastern Ditch**

Runoff = 36.91 cfs @ 12.82 hrs, Volume= 6.791 af, Depth= 3.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-47.99 hrs, dt= 0.03 hrs Type II 24-hr 25-yr, 24-hr Rainfall=6.66"

| Area (a     | ac) C         | N Des            | cription             |                   |                                                      |
|-------------|---------------|------------------|----------------------|-------------------|------------------------------------------------------|
| 20.3        | 390 7         | '4 >75°          | % Grass co           | over, Good        | , HSG C                                              |
| 0.7         | 750 9         | 6 Grav           | el surface           | , HSG C           |                                                      |
| 0.0         | )20 9         | 6 Grav           | /el surface          | , HSG C           |                                                      |
| 21.1        | 160 7         | '5 Wei           | ghted Aver           | age               |                                                      |
| 21.1        | 160           | 100.             | 00% Pervi            | ous Area          |                                                      |
| To          | Longth        | Clana            | Volocity             | Canacity          | Description                                          |
| Tc<br>(min) | Length (feet) | Slope<br>(ft/ft) | Velocity<br>(ft/sec) | Capacity<br>(cfs) | Description                                          |
| 15.5        | 200           | 0.0445           | 0.21                 | (013)             | Sheet Flow, Sheet Flow                               |
| 15.5        | 200           | 0.0445           | 0.21                 |                   | Grass: Short n= 0.150 P2= 2.04"                      |
| 57.0        | 1,221         | 0.0026           | 0.36                 |                   | Shallow Concentrated Flow, Shallow Concentrated Flow |
| 07.0        | 1,221         | 0.0020           | 0.00                 |                   | Short Grass Pasture Kv= 7.0 fps                      |
| 2.4         | 816           | 0.0057           | 5.72                 | 363.96            | Trap/Vee/Rect Channel Flow, Grassed Ditch            |
|             |               |                  |                      |                   | Bot.W=9.52' D=3.00' Z= 3.9 '/' Top.W=32.92'          |
|             |               |                  |                      |                   | n= 0.030 Earth, grassed & winding                    |
| 74.9        | 2,237         | Total            |                      |                   |                                                      |

#### **Subcatchment 1S: Northern Area - Eastern Ditch**



Prepared by TRC
HydroCAD® 10.00-25 s/n 01402 © 2019 HydroCAD Software Solutions LLC

Printed 10/7/2021

Page 8

## **Summary for Subcatchment 3S: Southern Area**

Runoff = 121.80 cfs @ 12.68 hrs, Volume= 20.142 af, Depth= 3.85"

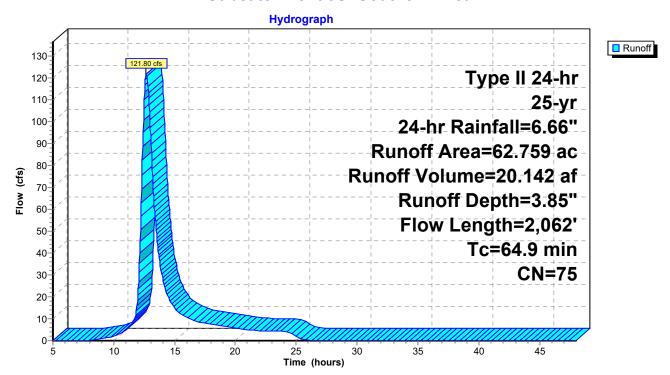
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-47.99 hrs, dt= 0.03 hrs Type II 24-hr 25-yr, 24-hr Rainfall=6.66"

| Area         | (ac) C | N Des   | cription    |            |                                                      |
|--------------|--------|---------|-------------|------------|------------------------------------------------------|
| 60.          | 715    | 74 >75° | % Grass co  | over, Good | , HSG C                                              |
| 1.           | 968    | 96 Grav | el surface  | , HSG C    |                                                      |
| 0.           | .076   | 96 Grav | /el surface | , HSG C    |                                                      |
| 62.          | 759    | 75 Weig | ghted Aver  | age        |                                                      |
| 62.          | 759    | 100.    | 00% Pervi   | ous Area   |                                                      |
|              |        |         |             |            |                                                      |
| Tc           | Length | Slope   | Velocity    | Capacity   | Description                                          |
| <u>(min)</u> | (feet) | (ft/ft) | (ft/sec)    | (cfs)      |                                                      |
| 15.1         | 196    | 0.0456  | 0.22        |            | Sheet Flow, Sheet Flow                               |
|              |        |         |             |            | Grass: Short n= 0.150 P2= 2.04"                      |
| 44.9         | 923    | 0.0024  | 0.34        |            | Shallow Concentrated Flow, Shallow Concentrated Flow |
|              |        |         |             |            | Short Grass Pasture Kv= 7.0 fps                      |
| 4.7          | 890    | 0.0028  | 3.19        | 110.30     | Trap/Vee/Rect Channel Flow, Grassed Ditch            |
|              |        |         |             |            | Bot.W=9.30' D=2.00' Z= 3.7 & 4.3 '/' Top.W=25.30'    |
|              |        |         |             |            | n= 0.030 Earth, grassed & winding                    |
| 0.2          | 53     | 0.0251  | 4.74        | 215.27     | Trap/Vee/Rect Channel Flow, Riprap Channel           |
|              |        |         |             |            | Bot.W=9.30' D=2.50' Z= 4.3 & 2.8 '/' Top.W=27.05'    |
|              |        |         |             |            | n= 0.069 Riprap, 6-inch                              |
| 64.9         | 2,062  | Total   |             |            |                                                      |

Prepared by TRC
HydroCAD® 10.00-25 s/n 01402 © 2019 HydroCAD Software Solutions LLC

Page 9

#### **Subcatchment 3S: Southern Area**



HydroCAD® 10.00-25 s/n 01402 © 2019 HydroCAD Software Solutions LLC

Page 10

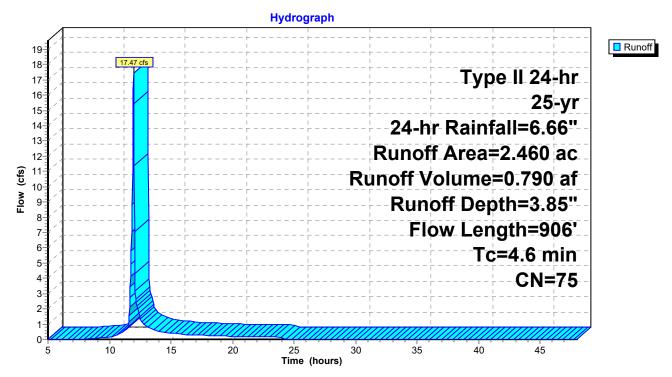
## Summary for Subcatchment 5S: Northern Area - Northern Ditch

Runoff = 17.47 cfs @ 11.96 hrs, Volume= 0.790 af, Depth= 3.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-47.99 hrs, dt= 0.03 hrs Type II 24-hr 25-yr, 24-hr Rainfall=6.66"

| _ | Area                      | (ac) C           | N Des            | cription             |                   |                                                   |  |  |  |  |  |  |
|---|---------------------------|------------------|------------------|----------------------|-------------------|---------------------------------------------------|--|--|--|--|--|--|
|   | 2.                        | 350 7            | '4 >75°          | % Grass c            | , HSG C           |                                                   |  |  |  |  |  |  |
| _ | 0.                        | 110 9            | 6 Grav           | el surface           | , HSG C           |                                                   |  |  |  |  |  |  |
|   | 2.460 75 Weighted Average |                  |                  |                      |                   |                                                   |  |  |  |  |  |  |
|   | 2.                        | 460              | 100.             | 00% Pervi            | ous Area          |                                                   |  |  |  |  |  |  |
|   | To                        | Longth           | Clana            | Volocity             | Canacity          | Description                                       |  |  |  |  |  |  |
|   | Tc<br>(min)               | Length<br>(feet) | Slope<br>(ft/ft) | Velocity<br>(ft/sec) | Capacity<br>(cfs) | Description                                       |  |  |  |  |  |  |
| - | 2.2                       | 34               | 0.1730           | 0.26                 | , ,               | Sheet Flow, Sheet Flow                            |  |  |  |  |  |  |
|   |                           |                  |                  |                      |                   | Grass: Short n= 0.150 P2= 2.04"                   |  |  |  |  |  |  |
|   | 2.4                       | 872              | 0.0089           | 6.11                 | 199.63            | Trap/Vee/Rect Channel Flow, Northern Ditch        |  |  |  |  |  |  |
|   |                           |                  |                  |                      |                   | Bot.W=9.29' D=2.15' Z= 2.6 & 2.9 '/' Top.W=21.11' |  |  |  |  |  |  |
| _ |                           |                  |                  |                      |                   | n= 0.030 Earth, grassed & winding                 |  |  |  |  |  |  |
|   | 4 6                       | 906              | Total            |                      |                   |                                                   |  |  |  |  |  |  |

#### **Subcatchment 5S: Northern Area - Northern Ditch**



HydroCAD® 10.00-25 s/n 01402 © 2019 HydroCAD Software Solutions LLC

Page 11

## **Summary for Reach 2R: Northern Discharge**

Inflow Area = 23.620 ac, 0.00% Impervious, Inflow Depth = 3.85" for 25-yr, 24-hr event

Inflow = 37.91 cfs @ 12.81 hrs, Volume= 7.581 af

Outflow = 37.88 cfs @ 12.85 hrs, Volume= 7.581 af, Atten= 0%, Lag= 2.3 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-47.99 hrs, dt= 0.03 hrs

Max. Velocity= 2.45 fps, Min. Travel Time= 1.4 min Avg. Velocity = 0.97 fps, Avg. Travel Time= 3.4 min

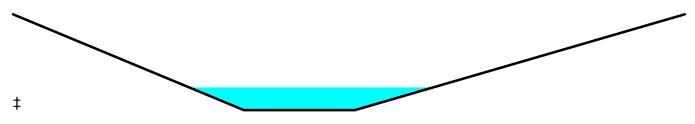
Peak Storage= 3,092 cf @ 12.83 hrs Average Depth at Peak Storage= 1.30'

Bank-Full Depth= 5.50' Flow Area= 143.6 sf, Capacity= 787.13 cfs

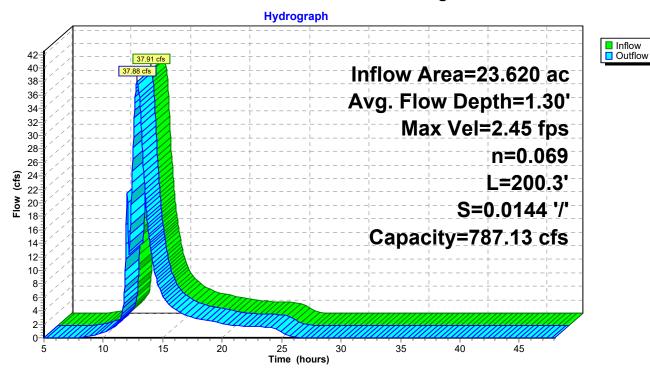
7.41' x 5.50' deep channel, n= 0.069 Riprap, 6-inch Side Slope Z-value= 2.8 4.0 '/' Top Width= 44.81'

Length= 200.3' Slope= 0.0144 '/'

Inlet Invert= 92.27', Outlet Invert= 89.39'



Reach 2R: Northern Discharge



HydroCAD® 10.00-25 s/n 01402 © 2019 HydroCAD Software Solutions LLC

Page 12

#### **Summary for Pond 7P: Box Culvert**

[57] Hint: Peaked at 97.19' (Flood elevation advised)

Inflow Area = 62.759 ac, 0.00% Impervious, Inflow Depth = 3.85" for 25-yr, 24-hr event

Inflow = 121.80 cfs @ 12.68 hrs, Volume= 20.142 af

Outflow = 121.80 cfs @ 12.68 hrs, Volume= 20.142 af, Atten= 0%, Lag= 0.0 min

Primary = 121.80 cfs @ 12.68 hrs, Volume= 20.142 af

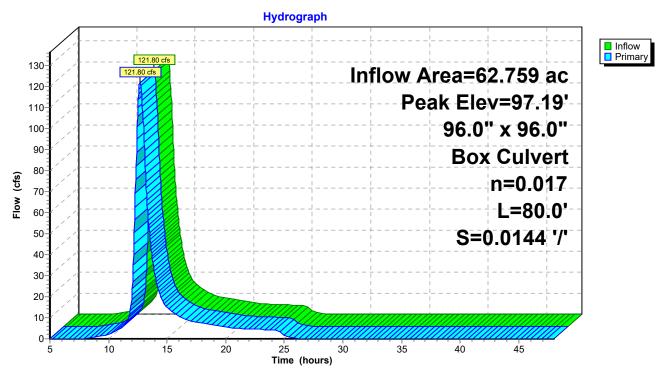
Routing by Stor-Ind method, Time Span= 5.00-47.99 hrs, dt= 0.03 hrs

Peak Elev= 97.19' @ 12.68 hrs

| Device | Routing | Invert | Outlet Devices                                                 |
|--------|---------|--------|----------------------------------------------------------------|
| #1     | Primary | 94.12' | 96.0" W x 96.0" H Box Box Culvert                              |
|        |         |        | L= 80.0' Box, 0° wingwalls, square crown edge, Ke= 0.700       |
|        |         |        | Inlet / Outlet Invert= 94.12' / 92.97' S= 0.0144 '/' Cc= 0.900 |
|        |         |        | n= 0.017 Concrete, unfinished, Flow Area= 64.00 sf             |

Primary OutFlow Max=121.77 cfs @ 12.68 hrs HW=97.19' (Free Discharge) 1=Box Culvert (Inlet Controls 121.77 cfs @ 4.96 fps)

## Pond 7P: Box Culvert



Prepared by TRC
HydroCAD® 10.00-25 s/n 01402 © 2019 HydroCAD Software Solutions LLC

Page 13

## **Summary for Link 5L: Site CFS**

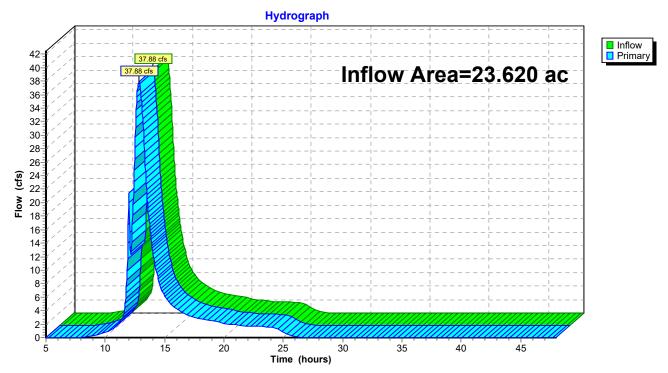
Inflow Area = 23.620 ac, 0.00% Impervious, Inflow Depth = 3.85" for 25-yr, 24-hr event

Inflow = 37.88 cfs @ 12.85 hrs, Volume= 7.581 af

Primary = 37.88 cfs @ 12.85 hrs, Volume= 7.581 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-47.99 hrs, dt= 0.03 hrs

#### Link 5L: Site CFS



Prepared by TRC
HydroCAD® 10.00-25 s/n 01402 © 2019 HydroCAD Software Solutions LLC

Page 14

## **Summary for Link 6L: Site CFS**

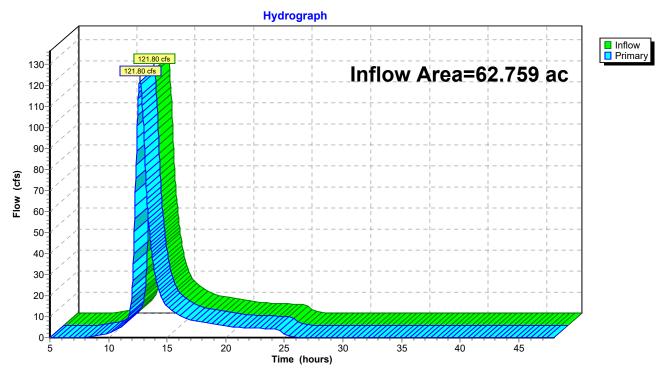
Inflow Area = 62.759 ac, 0.00% Impervious, Inflow Depth = 3.85" for 25-yr, 24-hr event

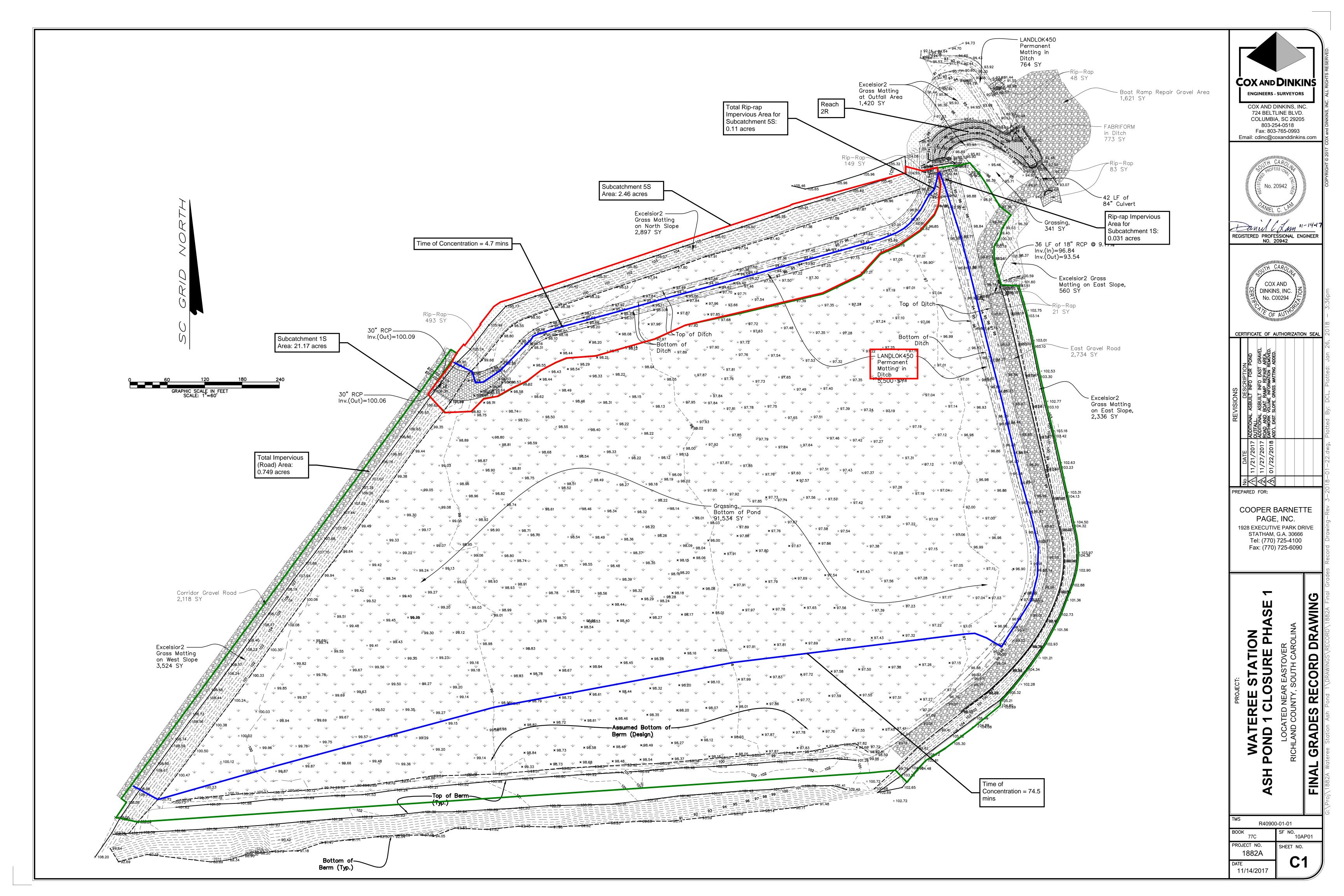
Inflow = 121.80 cfs @ 12.68 hrs, Volume= 20.142 af

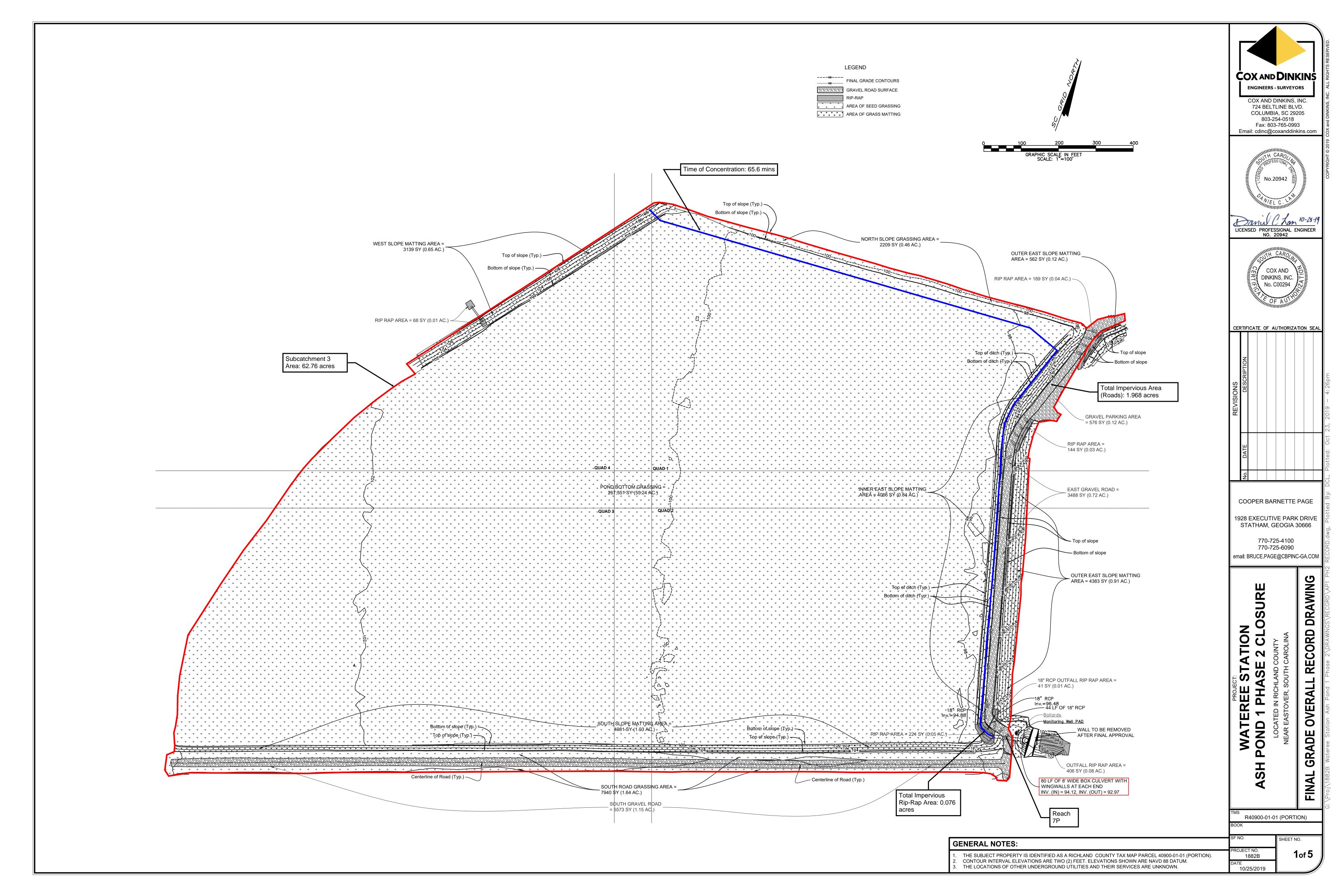
Primary = 121.80 cfs @ 12.68 hrs, Volume= 20.142 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-47.99 hrs, dt= 0.03 hrs

#### Link 6L: Site CFS









NOAA Atlas 14, Volume 2, Version 3 Location name: Eastover, South Carolina, USA\* Latitude: 33.8247°, Longitude: -80.6262° Elevation: 133.29 ft\*\*



source: ESRI Maps
\*\* source: USGS

#### POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M.Yekta, and D. Riley NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

#### PF tabular

| PDS      | PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) <sup>1</sup> |                            |                               |                            |                            |                            |                               |                           |                          |                          |
|----------|----------------------------------------------------------------------------------------------------------|----------------------------|-------------------------------|----------------------------|----------------------------|----------------------------|-------------------------------|---------------------------|--------------------------|--------------------------|
| Duration |                                                                                                          |                            |                               | Average                    | e recurrence               | interval (ye               | ars)                          |                           |                          |                          |
| Duration | 1                                                                                                        | 2                          | 5                             | 10                         | 25                         | 50                         | 100                           | 200                       | 500                      | 1000                     |
| 5-min    | <b>0.449</b><br>(0.416-0.484)                                                                            | <b>0.512</b> (0.476-0.552) | <b>0.577</b> (0.534-0.621)    | <b>0.659</b> (0.610-0.709) | <b>0.741</b> (0.682-0.797) | <b>0.818</b> (0.750-0.879) | <b>0.886</b><br>(0.808-0.952) | <b>0.954</b> (0.862-1.03) | <b>1.03</b> (0.926-1.12) | <b>1.12</b> (0.989-1.21) |
| 10-min   | <b>0.717</b><br>(0.664-0.774)                                                                            | <b>0.819</b> (0.761-0.882) | <b>0.924</b><br>(0.856-0.994) | <b>1.05</b> (0.975-1.13)   | <b>1.18</b> (1.09-1.27)    | <b>1.30</b> (1.19-1.40)    | <b>1.41</b> (1.28-1.51)       | <b>1.51</b> (1.37-1.63)   | <b>1.64</b> (1.47-1.76)  | <b>1.76</b> (1.56-1.90)  |
| 15-min   | <b>0.896</b><br>(0.830-0.967)                                                                            | <b>1.03</b> (0.956-1.11)   | <b>1.17</b> (1.08-1.26)       | <b>1.33</b> (1.23-1.43)    | <b>1.50</b> (1.38-1.61)    | <b>1.65</b> (1.51-1.77)    | <b>1.78</b> (1.62-1.91)       | <b>1.91</b> (1.72-2.05)   | <b>2.06</b> (1.84-2.22)  | <b>2.20</b> (1.96-2.38)  |
| 30-min   | <b>1.23</b> (1.14-1.33)                                                                                  | <b>1.42</b> (1.32-1.53)    | <b>1.66</b> (1.54-1.79)       | <b>1.93</b> (1.79-2.08)    | <b>2.22</b> (2.04-2.38)    | <b>2.48</b> (2.28-2.67)    | <b>2.73</b> (2.48-2.93)       | <b>2.97</b> (2.68-3.19)   | <b>3.28</b> (2.93-3.53)  | <b>3.57</b> (3.17-3.86)  |
| 60-min   | <b>1.53</b> (1.42-1.65)                                                                                  | <b>1.78</b> (1.66-1.92)    | <b>2.13</b> (1.97-2.29)       | <b>2.52</b> (2.33-2.71)    | <b>2.95</b> (2.72-3.17)    | <b>3.37</b> (3.09-3.62)    | <b>3.76</b> (3.42-4.04)       | <b>4.17</b> (3.76-4.48)   | <b>4.70</b> (4.21-5.07)  | <b>5.21</b> (4.62-5.64)  |
| 2-hr     | <b>1.73</b> (1.60-1.88)                                                                                  | <b>2.04</b> (1.88-2.21)    | <b>2.46</b> (2.27-2.66)       | <b>2.95</b> (2.72-3.19)    | <b>3.53</b> (3.24-3.81)    | <b>4.09</b> (3.73-4.40)    | <b>4.64</b> (4.21-4.99)       | <b>5.23</b> (4.71-5.63)   | <b>6.01</b> (5.36-6.49)  | <b>6.77</b> (5.98-7.34)  |
| 3-hr     | <b>1.82</b> (1.67-1.99)                                                                                  | <b>2.15</b> (1.98-2.35)    | <b>2.60</b> (2.39-2.84)       | <b>3.14</b> (2.89-3.42)    | <b>3.80</b> (3.47-4.13)    | <b>4.45</b> (4.04-4.82)    | <b>5.11</b> (4.59-5.53)       | <b>5.82</b> (5.19-6.30)   | <b>6.80</b> (6.00-7.38)  | <b>7.78</b> (6.77-8.45)  |
| 6-hr     | <b>2.15</b> (1.98-2.36)                                                                                  | <b>2.54</b> (2.35-2.78)    | <b>3.08</b> (2.83-3.36)       | <b>3.72</b> (3.41-4.06)    | <b>4.51</b> (4.12-4.92)    | <b>5.30</b> (4.81-5.76)    | <b>6.10</b> (5.48-6.62)       | <b>6.98</b> (6.21-7.57)   | <b>8.20</b> (7.20-8.91)  | <b>9.42</b> (8.15-10.2)  |
| 12-hr    | <b>2.52</b> (2.31-2.78)                                                                                  | <b>2.97</b> (2.73-3.27)    | <b>3.61</b> (3.31-3.97)       | <b>4.39</b> (4.01-4.81)    | <b>5.36</b> (4.87-5.86)    | <b>6.33</b> (5.71-6.91)    | <b>7.33</b> (6.55-7.99)       | <b>8.44</b> (7.45-9.19)   | <b>10.00</b> (8.69-10.9) | <b>11.6</b> (9.89-12.6)  |
| 24-hr    | <b>2.94</b> (2.71-3.23)                                                                                  | <b>3.54</b> (3.26-3.89)    | <b>4.51</b> (4.13-4.94)       | <b>5.35</b> (4.90-5.87)    | <b>6.66</b> (6.04-7.29)    | <b>7.81</b> (7.02-8.56)    | <b>9.12</b> (8.10-10.0)       | <b>10.6</b> (9.29-11.7)   | <b>12.9</b> (11.1-14.3)  | <b>14.9</b> (12.6-16.7)  |
| 2-day    | <b>3.42</b> (3.15-3.75)                                                                                  | <b>4.11</b> (3.79-4.50)    | <b>5.19</b> (4.77-5.69)       | <b>6.15</b> (5.63-6.74)    | <b>7.62</b> (6.92-8.36)    | <b>8.92</b> (8.03-9.81)    | <b>10.4</b> (9.25-11.5)       | <b>12.1</b> (10.6-13.4)   | <b>14.6</b> (12.6-16.3)  | <b>16.9</b> (14.2-19.0)  |
| 3-day    | <b>3.68</b> (3.40-4.01)                                                                                  | <b>4.41</b> (4.07-4.81)    | <b>5.53</b> (5.10-6.04)       | <b>6.51</b> (5.99-7.12)    | <b>8.00</b> (7.29-8.74)    | <b>9.30</b> (8.41-10.2)    | <b>10.8</b> (9.62-11.8)       | <b>12.4</b> (10.9-13.7)   | <b>14.9</b> (12.9-16.6)  | <b>17.1</b> (14.5-19.2)  |
| 4-day    | <b>3.93</b> (3.64-4.28)                                                                                  | <b>4.71</b> (4.36-5.13)    | <b>5.87</b> (5.43-6.39)       | <b>6.88</b> (6.34-7.49)    | <b>8.38</b> (7.67-9.13)    | <b>9.69</b> (8.79-10.6)    | <b>11.1</b> (10.00-12.2)      | <b>12.7</b> (11.3-14.0)   | <b>15.2</b> (13.2-16.8)  | <b>17.3</b> (14.8-19.3)  |
| 7-day    | <b>4.59</b> (4.27-4.96)                                                                                  | <b>5.48</b> (5.09-5.93)    | <b>6.74</b> (6.25-7.29)       | <b>7.80</b> (7.22-8.44)    | <b>9.33</b> (8.58-10.1)    | <b>10.6</b> (9.71-11.5)    | <b>12.0</b> (10.9-13.0)       | <b>13.5</b> (12.1-14.7)   | <b>15.8</b> (13.9-17.3)  | <b>17.8</b> (15.6-19.7)  |
| 10-day   | <b>5.24</b> (4.90-5.62)                                                                                  | <b>6.23</b> (5.83-6.69)    | <b>7.57</b> (7.06-8.13)       | <b>8.67</b> (8.08-9.30)    | <b>10.2</b> (9.48-11.0)    | <b>11.5</b> (10.6-12.4)    | <b>12.8</b> (11.8-13.8)       | <b>14.3</b> (13.0-15.4)   | <b>16.4</b> (14.7-17.8)  | <b>18.3</b> (16.2-20.1)  |
| 20-day   | <b>7.01</b> (6.56-7.53)                                                                                  | <b>8.29</b> (7.75-8.91)    | <b>9.87</b> (9.22-10.6)       | <b>11.2</b> (10.4-12.0)    | <b>13.0</b> (12.1-14.0)    | <b>14.5</b> (13.4-15.6)    | <b>16.0</b> (14.7-17.3)       | <b>17.6</b> (16.0-19.1)   | <b>19.8</b> (17.9-21.6)  | <b>21.5</b> (19.2-23.7)  |
| 30-day   | <b>8.64</b> (8.11-9.23)                                                                                  | <b>10.2</b> (9.55-10.9)    | <b>11.9</b> (11.2-12.8)       | <b>13.3</b> (12.5-14.3)    | <b>15.2</b> (14.2-16.3)    | <b>16.6</b> (15.5-17.8)    | <b>18.1</b> (16.7-19.4)       | <b>19.5</b> (18.0-21.0)   | <b>21.4</b> (19.6-23.2)  | <b>22.9</b> (20.8-24.9)  |
| 45-day   | <b>10.8</b> (10.2-11.5)                                                                                  | <b>12.7</b> (11.9-13.5)    | <b>14.7</b> (13.8-15.6)       | <b>16.2</b> (15.2-17.3)    | <b>18.3</b> (17.2-19.5)    | <b>19.9</b> (18.6-21.3)    | <b>21.5</b> (20.0-23.0)       | <b>23.1</b> (21.4-24.7)   | <b>25.1</b> (23.1-27.1)  | <b>26.7</b> (24.4-28.9)  |
| 60-day   | <b>12.9</b> (12.2-13.6)                                                                                  | <b>15.1</b> (14.2-16.0)    | <b>17.3</b> (16.3-18.3)       | <b>19.0</b> (17.9-20.1)    | <b>21.2</b> (19.9-22.5)    | <b>22.9</b> (21.4-24.3)    | <b>24.5</b> (22.9-26.1)       | <b>26.1</b> (24.3-27.8)   | <b>28.1</b> (26.0-30.1)  | <b>29.5</b> (27.2-31.7)  |

<sup>&</sup>lt;sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

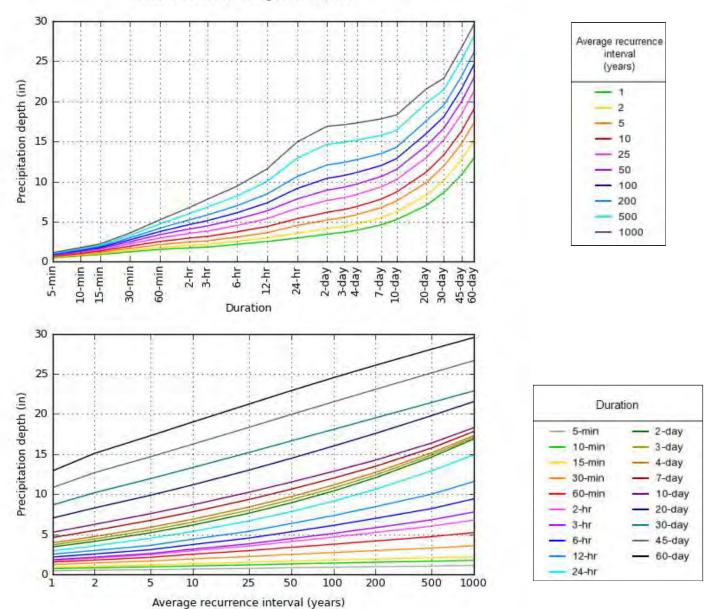
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

Back to Top

#### PF graphical

#### PDS-based depth-duration-frequency (DDF) curves Latitude: 33.8247°, Longitude: -80.6262°



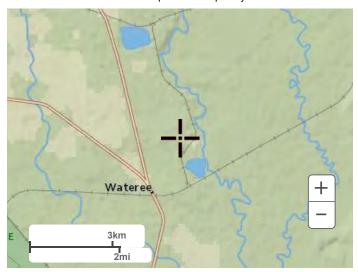
NOAA Atlas 14, Volume 2, Version 3

Created (GMT): Thu Sep 9 07:42:47 2021

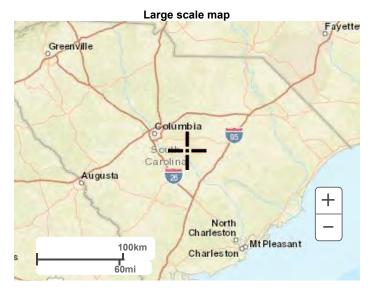
Back to Top

#### Maps & aerials

Small scale terrain







Large scale aerial



Back to Top

US Department of Commerce

National Oceanic and Atmospheric Administration

National Weather Service

National Water Center

1325 East West Highway

Silver Spring, MD 20910

Questions?: HDSC.Questions@noaa.gov

**Disclaimer**